

COMPARISON OF VACUUM AND CENTRIFUGE-BASED TECHNIQUES FOR
EVALUATING SOLUTE TRANSPORT PROCESSES UNDER UNSATURATED
CONDITIONS

by

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(Under the Direction of David E. Radcliffe)

ABSTRACT

Most subsurface contamination passes through the unsaturated zone before reaching an aquifer; however, many transport studies are conducted under saturated conditions that may not approximate the natural system. Chromate migration was measured in sediment from the Savannah River Site, SC under different water contents using vacuum and centrifuge techniques to obtain a steady-state flow regime. Leaching solutions contained 0.5 or 1.0 mM Cr(VI) and tritium in artificial groundwater. Breakthrough curves were modeled using CXTFIT assuming equilibrium conditions. Dispersion, as indicated by Peclet number, increased with decreasing water content. Retardation increased with decreasing water content with no trend evident when K_d was calculated from R . Average K_d of all Cr(VI) experiments was $0.551 \text{ mL}\cdot\text{g}^{-1}$, similar to K_d derived from batch equilibration ($0.599 \text{ mL}\cdot\text{g}^{-1}$). Though results in both systems were similar, experiment duration in the vacuum system was 4 to 23 times longer than in the centrifuge system at comparable water contents.

INDEX WORDS: Hexavalent chromium, Solute transport, Unsaturated soil, Adsorption, Dispersion, Unsaturated flow apparatus

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DEDICATION

I would like to dedicate this thesis to my parents, Bob and Kim Hutchison. It is their love, perseverance, and support that have always inspired me. I love you both very much. Thanks for everything you do.

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TABLE OF CONTENTS

	Page
ACKNOWLEDGEMENTS	v
LIST OF TABLES	viii
LIST OF FIGURES	x
CHAPTER	
1 INTRODUCTION AND LITERATURE REVIEW.....	1
Rationale.....	1
Transport equation.....	1
Variation in solute transport parameters	8
Methods and equipment used to evaluate solute transport.....	13
Tracers.....	18
Modeling	20
Objectives	21
Notation	22
References.....	22
2 SOLUTE TRANSPORT IN VARIABLY-SATURATED SOIL	29
Abstract.....	30
Introduction	31
Materials and methods.....	35
Results and discussion	43

Summary and conclusions.....	57
Notation	58
References.....	58
3 CONCLUSIONS.....	84
APPENDICES.....	86

LIST OF TABLES

	Page
Table 1: Review of studies evaluating changes in transport parameters with water content.....	62
Table 2: Definitions of transport parameters and the effect of the variation of water content	63
Table 3: Physical and chemical characteristics of Tobacco Road sand used in column and batch studies	64
Table 4: Bromide transport parameters determined by direct measurement or from solute breakthrough curves evaluated using the equilibrium and nonequilibrium transport models	65
Table 5: Tritium transport parameters for vacuum system, derived by direct measurement or from solute breakthrough curves evaluated using the equilibrium transport model.....	66
Table 6: Tritium transport parameters for the UFA (centrifuge system), 0.5 mM Cr(VI) tracer solution, determined using the equilibrium transport model	67
Table 7: Tritium transport parameters for the UFA (centrifuge system), 1.0 mM Cr(VI) tracer solution, determined using the equilibrium transport model	68
Table 8: Cr(VI) transport parameters for vacuum system, derived by direct	

measurement or from solute breakthrough curves evaluated using the
equilibrium transport model..... 69

Table 9: Cr(VI) transport parameters for the UFA (centrifuge system), 0.5 mM
Cr(VI) tracer solution, determined using the equilibrium transport
model 70

Table 10: Cr(VI) transport parameters for the UFA (centrifuge system), 1.0 mM
Cr(VI) tracer solution determined using the equilibrium transport
model 71

Table 11: Amount of Cr(VI) sorbed by soil during the course of the
experiment 72

LIST OF FIGURES

	Page
Figure 1: Unsaturated hydraulic conductivity curves for sandy loam soil obtained using the UFA with artificial groundwater (AGW) and calcium chloride leaching solutions.....	73
Figure 2: Diagram of the Wierenga column setup for unsaturated flow	74
Figure 3: Diagram of the Unsaturated Flow Apparatus components	75
Figure 4: Cr(VI) sorption isotherm on loamy sand soil at a soil:solution ratio of 1:6 by mass, equilibrated for 24 hours	76
Figure 5: Bromide breakthrough curves for four levels of column saturation.	77
Figure 6: Volumetric and gravimetric water content with column length for vacuum column experiment under 50 cbars of vacuum pressure	78
Figure 7: Retardation in unsaturated media determined using centrifuge-based (UFA) and vacuum-based column systems	79
Figure 8: Log of Peclet number with volumetric water content for vacuum and UFA tritium column data	80
Figure 9: Log of dispersivity with water content for tritium data from vacuum and centrifuge column experiments, determined from D fit with equilibrium model.....	81

Figure 10: Retardation factor (graph A) and distribution coefficient
(graph B) with volumetric water content for UFA and vacuum
column systems at 0.5 mM and 1.0 mM Cr(VI) concentration
levels..... 82

Figure 11: Amount of Cr(VI) sorbed by soil in UFA and vacuum column
experiments with volumetric water content (graph A) and retention
time of solute in soil column (graph B) 83

CHAPTER 1

INTRODUCTION AND LITERATURE REVIEW

Rationale

Studies evaluating solute transport processes in the vadose zone yield important information for situations in which chemical wastes are disposed of at or below the land surface. Despite the fact that virtually all subsurface contamination must pass through the unsaturated vadose zone before reaching an aquifer, most studies have only focused on processes occurring under saturated conditions (Thomas and Swoboda, 1970; Passioura and Rose, 1971; Nkedi-Kizza et al., 1983; Powers et al., 1992; Jardine et al., 1999). This is partially due to the difficulty in maintaining steady-state flow and moisture content necessary for the controlled study of solute transport processes under unsaturated conditions. Furthermore, measurements made under saturated conditions may not approximate the conditions of the natural system under which solute transport occurs.

Transport equation

The convective-dispersive equation (CDE) (Lapidus and Amundson, 1952) was developed in an attempt to quantify solute transport processes for one-dimensional steady-state flow of a non-reactive solute in a homogeneous soil column and is described by the equation:

$$\frac{\partial C}{\partial t} = D \frac{\partial^2 C}{\partial z^2} - v \frac{\partial C}{\partial z} \quad [1]$$

where C is the concentration ($M \cdot L^{-3}$), D is the hydrodynamic dispersion coefficient ($L^2 \cdot T^{-1}$), v is the average pore-water velocity ($L \cdot T^{-1}$), z is distance, and t is time.

In order for this equation to be valid, it is assumed that all the fluid participates in the transport process and no interaction occurs between the solute and the soil matrix.

Dispersion

The hydrodynamic dispersion coefficient (D) has been shown to be uniquely related to the product of the average pore-water velocity (v) and the aggregate size (Passioura, 1971; Passioura and Rose, 1971) and results from the non-uniformity of flow velocity in the soil's conducting pores. This causes mixing to occur within the soil pores and results in the deviation of solute breakthrough curves (BTCs) from piston flow. Hydrodynamic dispersion is expressed as (Bear, 1969):

$$D = \lambda \cdot v^a + D_e \quad [2]$$

where λ and a are constants, λ is referred to as dispersivity (L) and is a characteristic property of the porous media, a is taken as a value between 1 and 2 (Freeze and Cherry, 1979), and D_e is the effective molecular diffusion coefficient through the media. The product $\lambda \cdot v^a$ in Equation [2] is referred to as mechanical dispersion. This is caused by kinematic and dynamic mechanisms (Sahimi et al., 1983). The kinematic mechanism results from a variation in the length of the streamlines that traverse the length of the column, while the dynamic mechanism results from a variation in the speed of fluid movement from one streamline to the next.

The second term on the right-hand side of Equation [2], D_e , is defined as (Shackelford, 1991):

$$D_e = \tau \cdot D_0 \quad [3]$$

where D_0 is the molecular diffusion coefficient in free solution and τ is the tortuosity factor $(L \cdot L_e^{-1})^2$, where L is the column length and L_e is the actual flow path length through the soil column. As the soil is desaturated, the actual length of the flow path (L_e) increases, causing the tortuosity factor to decrease. This causes the effective molecular diffusion coefficient (D_e) to be smaller in media with more tortuous flow paths. Burdine (1953) experimentally derived a relationship that relates the tortuosity factor for unsaturated media (τ_s) to the degree of saturation (S). This relationship is given as:

$$\tau_s = \tau_{S=1} + S_e^2 \quad [4]$$

where $\tau_{S=1}$ is the tortuosity factor when the media is fully saturated and S_e is the effective saturation [$S_e = (S - S_r) \cdot (1 - S_r)^{-1}$, where S_r is the residual saturation of the media, i.e. the minimum saturation that can be attained by dewatering a soil under increasing suction].

At high pore-water velocities, the contribution of molecular diffusion (D_e) to dispersion (D) is negligible and, therefore, often ignored in the calculation of hydrodynamic dispersion.

In addition to the effects of dispersion, longitudinal spreading of solutes in porous media may also be due to the existence of physical or chemical nonequilibrium processes during solute transport. This spreading should not be included in the dispersion coefficient if it is to be referred to as the hydrodynamic

dispersion coefficient. These nonequilibrium processes, in part, are the result of physical or chemical nonuniformity and sometimes cannot be independently measured or separated from the effects of other processes occurring within the media.

For comparison between systems, a dimensionless dispersion parameter, known as the Peclet number (P) is calculated as:

$$P = \frac{v \cdot L}{D} \quad [5]$$

where v is the average pore water velocity (Darcy flow velocity divided by volumetric water content), L is the column length, and D is the hydrodynamic dispersion coefficient. The Peclet number can be interpreted as the ratio of the characteristic times for hydrodynamic dispersion ($L^2 \cdot D^{-1}$) and convection ($L \cdot v^{-1}$) and is solely dependent on the medium, provided the contribution to mixing by molecular diffusion is insignificant (Nkedi-Kizza et al., 1983). Another parameter calculated from D is the dispersivity (λ), which is defined as the hydrodynamic dispersion coefficient (D) divided by the average pore water velocity (v).

Equation [1] is limited to describing solutes that exhibit conservative, or nonreacting, behavior. Reactive solutes can either be adsorbed by the soil (Hamaker, 1975) or electrostatically excluded from a fraction of the saturated pore space (McMahon and Thomas, 1974). Equation [1] also does not account for the fact that part of the soil solution may be stagnant and not participate in the flow process (Krupp et al., 1972; Gupta et al., 1973). Changes to Equation [1] to account for these phenomena include the retardation coefficient and the mobile-immobile water theory discussed in the text to follow.

Retardation

In addition to dispersion, the divergence of BTCs from piston flow can also result from the interaction of the solute with the media. There are two types of experiments designed to evaluate the chemical interactions of these solutes: batch equilibration and dynamic column tests. The distribution coefficient (K_d) is a specific type of batch equilibration study and is defined as:

$$K_d = \frac{(x/m)}{C} \quad [6]$$

where x is the mass of the adsorbed solute, m is the mass of the sorbent, and C is the solute concentration. The distribution coefficient (K_d) can also be estimated as the slope of the adsorption isotherm at the origin (Langmuir, 1997).

In dynamic column studies, one observes the retardation factor (R) for a given solute, defined as the ratio of the solution velocity to that of the solute. For linear, equilibrium adsorption R can be related to K_d through the expression (Bouwer, 1991):

$$R = \frac{V_{gw}}{V_{sp}} = 1 + \rho_b \frac{K_d}{\theta} \quad [7]$$

where V_{gw} is the velocity of the carrier fluid, V_{sp} is the velocity of the solute species, ρ_b is the dry bulk density, and θ is the water content. This expression is used when converting a K_d measurement derived from batch experiments to R values expected for a column experiment. However, retardation coefficients observed in column studies often differ from values determined by batch equilibration.

If the solute is nonreactive, i.e. $K_d = 0$, then $R = 1$ for that species and the solute will travel with the water at the same transport velocity (Hillel, 1980). Therefore, a solute exhibiting exclusion will have an $R < 1$ and a solute that is retained by the media will have an $R > 1$.

Retardation factors affect the calculation of parameters in the CDE.

When including a term for reactive solutes the expression becomes:

$$R \frac{\partial C}{\partial t} = D \frac{\partial^2 C}{\partial z^2} - v \frac{\partial C}{\partial z} \quad [8]$$

to include the retardation factor (R). Non-reactive tracers are used to estimate the physical transport parameters under the assumption that no chemical reactions occur between the solute and porous medium (Seaman et al., 1995). With $R = 1$, the CDE is calibrated using the BTC of the tracer to determine the transport parameters v and D (Parker and van Genuchten, 1984). The sorbing solute BTC is then fit with the CDE using the calibrated D and v values from the conservative tracer to find its appropriate R value.

Mobile-immobile water

In addition to complications resulting from sorption and exclusion, researchers have shown nonsigmoid and asymmetrical BTCs, which have been referred to as “tailing,” and contradict predictions from Equation [1] (Nielsen and Biggar, 1961; Krupp and Elrick, 1968; Gaudet et al., 1977; De Smedt and Wierenga, 1984).

Tailing has been observed under several conditions. Nielsen and Biggar (1961) noted considerable tailing with decreasing water content at approximately the same flow velocity. They attributed this effect to the fact that under

unsaturated conditions, the larger pores have drained and are eliminated for transport, thus increasing the portion of water which does not readily move within the soil, often referred to as dead, stagnant, or immobile water (Turner, 1958; Deans, 1963; Coats and Smith, 1964). Immobile water can exist as liquid films around soil particles, in dead-end pores, or in isolated regions associated with desaturation (Nielsen et al., 1986). Evidence of the presence of immobile regions of water can be inferred from the tailing and asymmetry of solute BTCs (Bond and Wierenga, 1990).

Another situation under which tailing has been observed is with aggregated media (Nkedi-Kizza et al., 1983). This effect has been noticed in soils even under saturated conditions and results from the fact that aggregates contain micropores in which displacement is dependent upon diffusion, since convection in these smaller pores is usually negligible. This results in slow and incomplete mixing and produces a BTC that exhibits tailing. In large aggregates, the amount of immobile water is greater, while the diffusion pathway becomes larger, resulting in BTCs with more extensive tailing (Biggar and Nielsen, 1962; Green et al., 1972; McMahon and Thomas, 1974). The magnitude of such a phenomenon depends on aggregate dimensions and the relative fluid velocities within and around the aggregates (Smiles et al., 1981).

Solute migration through materials with immobile water can be described by a "two-region" model, where advective transport is limited to the mobile water domain and transport between the mobile and immobile domains is diffusion-limited. Besides v , D , and R , the fraction of mobile water (β) and the mass

transfer coefficient (ω) parameters are fit to the BTCs. The fraction of mobile water ranges between 0 and 1 and is defined as:

$$\beta = \frac{\theta_m}{\theta} \quad [9]$$

where θ_m is the mobile fraction of the volumetric water content. Therefore, in situations where $\beta = 1$, the system is at physical equilibrium and would be well-described using the conventional CDE. As β decreases, the solute BTCs become more skewed and may exhibit early breakthrough and tailing. The mass transfer coefficient (ω) includes an exchange rate coefficient and is defined as:

$$\omega = \frac{\gamma \cdot L}{J_w} \quad [10]$$

where γ is the exchange rate coefficient between the mobile and immobile water regions, L is the column length, and J_w is the volumetric water flux. When ω is large, solute BTCs are relatively symmetrical even if there is a significant amount of immobile water in the system and are well described using the conventional CDE. Physical equilibrium conditions exist due to the rapid exchange between regions of mobile and immobile water. Also, at small water fluxes, solute residence times are often sufficient to ensure complete mixing between mobile and immobile water regions, such that all the water in the system appears mobile when estimated from solute BTCs (Nkedi-Kizza et al., 1983).

Variation in solute transport parameters

Differences in moisture content can influence solute migration by limiting diffusion and decreasing the pore space available for transport. Due to capillary forces, small pores tend to remain water-filled, while larger pores drain readily.

This increases the fraction of immobile water and may increase dispersion (Nielsen and Biggar, 1961; Gaudet et al., 1977; De Smedt and Wierenga, 1984; Maciejewski, 1993). In addition, the degree of saturation may also influence the availability of surface sites for reaction with the transported solutes, influencing the apparent retardation. Therefore, it is important to study the effect of moisture content on various parameters used in predicting solute transport.

Dispersion

Hydrodynamic dispersion is accountable for the divergence of breakthrough curves (BTCs) from ideal piston flow (Hillel, 1980). Application of the capillary tube model of Aris (1956) would predict, for a given average pore-water velocity, the dynamic part of mechanical dispersion would decrease as the media is desaturated. This results from the reduction in the possible range of pore sizes as the larger pores are emptied, but this concept may be more applicable to structured soils. Also, assumptions made from the capillary tube model are limited in their application to soils since capillary tubes do not intersect each other and cannot account for the increase in the tortuosity of the solute flow path with desaturation (Maraqa et al., 1997). However, the data of James and Rubin (1986) agree with the predictions of the capillary tube model, showing a value for dispersivity in an unsaturated column to be three times smaller than in the saturated case. However, preliminary column tritium data for sandy soil suggest an increase in hydrodynamic dispersion with decreasing saturation (Seaman et al., 2002). Results from other researchers have also shown an increase in dispersion with decreasing saturation (Biggar and Nielsen, 1976; Yule

and Gardner, 1978; De Smedt and Wierenga, 1979, 1984; De Smedt et al., 1986; Maraqa et al., 1997; Fesch et al., 1998; Jin et al., 2000; Gamedainger and Kaplan, 2001). De Smedt and Wierenga (1984) observed that porous media at 90% saturation or above displayed identical dispersive behavior to fully saturated materials.

De Smedt and Wierenga (1979) assumed that hydrodynamic dispersion retains the same value, at a given pore water velocity, regardless of the saturation of the media. They attributed the increase in dispersion of their unsaturated experiments to the existence of a physical nonequilibrium situation created by solute exchange between mobile and immobile water regions. Yule and Gardner (1978), however, reported symmetrical BTCs with what appears to be mechanical dispersion in unsaturated media that is higher than under saturated conditions. They speculated that this occurred because of a wider variation in the pore water velocity distribution when the soil is desaturated. Maraqa et al. (1997) found dispersivity for tritium under unsaturated conditions to be higher than the value for saturated media, while De Smedt et al. (1986) found dispersivity to be 78 times greater for unsaturated sand as opposed to saturated sand. An experiment with the non-reactive tracer, thiourea, yielded an increase in dispersivity (0.1 cm to 1.3 cm) with decreasing volumetric water content (0.319 to 0.168) (Fesch et al., 1998). They attributed this to the air-filled pore space increasing the tortuosity of the solute flow path through the column. Jin et al. (2000) found an increase in the dispersion coefficient from 0.91 to 21.78 $\text{cm}^2 \cdot \text{hr}^{-1}$ for bromide in Ottawa sand with a decrease in saturation from 100 to 21.4%,

respectively. Although not explicitly stated in Porro et al. (2000), it can be inferred from Peclet numbers calculated from their data, that hydrodynamic dispersion increased with decreasing saturation for crushed basalt.

Magesan et al. (1995) found that dispersivity was higher for intact soil columns with weaker structure. They indicated that dispersivity may also be a useful length scale for the assessment of soil structure.

Retardation

One of the many concerns surrounding the use of distribution coefficients is the effect of soil moisture content. Batch studies for determining distribution coefficients are often conducted at solid:liquid ratios much lower than those found under natural conditions and may underestimate sorption due to the solids effect (Celorie et al., 1989). A decrease in the distribution coefficient with an increase in solids concentration has been observed for phenol on kaolinite and strontium on crushed basalt (Celorie et al., 1989; Porro et al., 2000). This effect may be due to the fact that, as the solid:liquid ratio increases, the effective surface area accessible to solutes decreases due to particle-particle interactions. However, the solids effect may also be explained by the inability to separate the liquid and solid phases as the solid:liquid ratio increases. Therefore, an apparent decrease in the distribution coefficient is observed, resulting from the analysis of solute bound to suspended colloids in the sample present in the "liquid" phase.

Lindenmeier et al. (1995) found that K_d for U and Sr decreased 70% and 54%, respectively, with a decrease in saturation from 100% to about 28%. Uranium sorption has been shown to decrease with decreasing water content,

due to rate-limited adsorption effects and faster local flow velocities present through the mobile water regions (Gamerding et al., 2001a; Gamerding et al., 2001b). Fesch et al. (1998) found an increase in retardation with decreasing water content for an organic reactive tracer. They attributed this increase to the relative increase of the number of sorption sites per water volume as the media is desaturated.

Mobile-immobile water

The CDE may not accurately describe the physical processes that occur in aggregated materials (Biggar and Nielsen, 1962; Green et al., 1972; van Genuchten and Wierenga, 1977) and unsaturated media (Nielsen and Biggar, 1961; Gupta et al., 1973; Gaudet et al., 1977; De Smedt et al., 1986). A discussion of the impact of aggregation on saturated flow is given in Passioura (1971) and Passioura and Rose (1971), but the effects of aggregation and immobile water are still an issue of dispute in unsaturated soils. Gaudet et al. (1977) found a 4-40% increase in the relative amount of stagnant water with a decrease in water content from 71-55%. A decrease in water content increases the fraction of air-filled macro-pores, resulting in the creation of additional dead-end or blind pores which rely on diffusion to attain equilibrium with the displacing solution (Fatt et al., 1960; Gupta et al., 1973).

Tailing, an indication of the presence of immobile water, has been reported to become more pronounced with an increase in pore water velocity (Biggar and Nielsen, 1962; Skopp and Warrick, 1974; Nkedi-Kizza et al., 1983). This could be due to the fact that with larger fluxes, the residence time of the

solute in the column is small, not allowing sufficient mixing to occur between the mobile and immobile water regions. With smaller fluxes, the residence time is greater, allowing complete mixing and the appearance that all the water in the column is mobile (Nkedi-Kizza et al., 1983).

Methods and equipment used to evaluate solute transport

Two types of experiments are designed to evaluate the chemical interactions of solutes and media: batch equilibration and dynamic column tests. Batch tests are often implemented because they are relatively simple and fast compared to conducting conventional column tests, even though column experiments more closely approximate the solid:liquid ratio observed in natural systems.

Column methods are effective tools for investigating solute transport mechanisms in porous media. Most conventional column studies have been performed under saturated conditions for measurement of hydraulic conductivity and solute transport processes. Transport parameters measured under saturated conditions are often extrapolated to unsaturated systems under the assumption that processes controlling retardation are consistent. A significant limitation of using conventional column methods is the time required in evaluating fine-textured soils with low hydraulic conductivities (Celorie et al., 1989).

An alternative to conventional saturated systems is the use of columns under vacuum to maintain unsaturated steady-state flow, as in the commonly used Wierenga system. This technique utilizes tensiometers installed at both ends of the column to monitor matric potential due to the vacuum that is applied

at the bottom of the column. The vacuum is controlled with a pressure regulator until the tension along the column has equalized (van Genuchten and Wierenga, 1986). Disadvantages to using this technique include the inability to achieve a constant moisture content below 50% of saturation, the long experimental time required for a single breakthrough experiment, and the potential for mechanical failure due to laboratory power outages during the extended testing period (Lindenmeier et al., 1995).

Centrifuge techniques have also been employed to simulate unsaturated flow through geologic materials and have the advantage of being able to obtain stable, low water contents in a relatively short period of time, even for fine-grained materials (Gamerding and Kaplan, 2000). These techniques use centrifugal force and inlet flow rates to achieve reproducible steady-state unsaturated flow conditions (Nimmo and Mello, 1991). In the centrifuge, soil columns are spun at high rotational speeds that increase the gravitational effects and simulate an *in situ* confining stress (Celorie et al., 1989).

The centrifuge technique, initially described by Nimmo et al., (1987; 1992), was modified by researchers at Pacific Northwest National Laboratory (PNNL) (Conca and Wright, 1992) to include an ultracentrifuge, rotor, and rotating seal, which made it possible to add a treatment solution simultaneously to two soil columns at a controlled rate while the rotor was spinning. The modified centrifuge is referred to as an unsaturated flow apparatus or UFA[®] (model L8-UFA, Beckman Coulter, Inc., Fullerton, CA).

Centrifuge systems have been widely used in geosciences with limited application to evaluating flow and transport processes (Celorie et al., 1989; Gamerdinger and Kaplan, 2000). Centrifugal force has been used to measure the hydraulic conductivity of fine-textured materials (Nimmo et al., 1987; Nimmo and Akstin, 1988; Nimmo and Mello, 1991; Nimmo et al., 1992; Khaleel et al., 1995), aquifer recharge rates (Nimmo et al., 1994), sorption equilibrium distribution coefficients (Celorie et al., 1989), and other steady-state hydraulic properties. This method has also been employed in obtaining unsaturated hydraulic conductivity measurements (Nimmo et al., 1987; Nimmo et al., 1992; Khaleel et al., 1995) as well as saturated hydraulic conductivity measurements for low permeability materials (Nimmo and Mello, 1991). The UFA centrifuge system can be used to easily simulate moisture conditions as low as 25% of saturation (Lindenmeier et al., 1995).

Centrifugation techniques have been implemented in determining distribution coefficients in fine-grained soils at solids concentrations representative of those occurring in natural systems, avoiding the “solids effect” complication observed when using batch equilibration methods (Celorie et al., 1989). In addition, hydraulic conductivity measurements with this technique have been made as low as $1.1 \times 10^{-11} \text{ m}\cdot\text{s}^{-1}$ at $0.068 \text{ m}^3\cdot\text{m}^{-3}$ water content for a sandy soil (Nimmo et al., 1992).

Possible problems and disadvantages associated with centrifuge column methods are maintaining stable water content along the length of the column, physical compaction problems due to the centrifugal forces, and disruption of

solute transport processes during necessary stoppage for collection of effluent samples.

Water content has been reported as stable throughout the soil column in centrifuge experiments except for the final segment at the outflow, in contact with the water-absorbent filter paper (Gamerding and Kaplan, 2000).

Soil compaction has been noted when using centrifugation techniques (Khaleel et al., 1995), but has mainly been limited to fine-textured sediments (Nimmo et al., 1994; Gamerding and Kaplan, 2000; Gamerding et al., 2001b). Soil compaction due to the centrifuge technique has been observed to effect the transport of sorbing solutes (Gamerding et al., 2001a), but not non-sorbing solutes (Gamerding and Kaplan, 2000).

Gamerding and Kaplan (2000) compared BTCs generated with centrifuge techniques to conventional saturated columns for non-sorbing solutes. They found little difference in solute transport behavior, indicating that stopping the centrifuge for effluent sampling had little effect because minimal water redistribution occurred within the unsaturated soil in the absence of the centrifugal force. Changes in the behavior of sorbing solutes, however, can be attributed to the increased reaction time when flow is interrupted (Gamerding et al., 2001a; Gamerding et al., 2001b).

It has also been of concern whether the CDE is appropriate for columns of short length such as those used in the UFA. However, Magesan et al. (1995) found that chloride breakthrough in columns 3.7 cm long and 7.3 cm in diameter were adequately described by the CDE. Based on this finding, the BTCs

generated with the 5-cm long UFA columns in this study should be adequately described by the CDE.

Since few studies have focused on unsaturated solute transport, the Wierenga and the UFA systems present an opportunity to examine transport parameters under a range of moisture contents. Limited data have been obtained comparing unsaturated transport in these two systems (Lindenmeier et al., 1995).

Effect of sample preparation

Variation in solute sorption as a function of water content may be indicative of the physical distribution of reactive mineral surfaces within the column. McMahon and Thomas (1974) found that physical disruption in repacked soil columns significantly affected migration and retardation of tracers compared to undisturbed columns. Sample preparation and treatment has been shown to alter bromide retardation due to the exposure of fresh mineral surfaces resulting from particle abrasion and fracturing during sample collection or changes in particle surface chemistry from the drying of soils prior to use in columns (Boggs and Adams, 1992; Seaman et al., 1995). The disruption of a soil by air-drying and sieving caused a slight decrease in the apparent dispersion coefficient compared to undisturbed loam soil cores (Cassel et al., 1974).

The degree of saturation in each column may affect the number and type of surfaces that are available for sorption. Desaturation of the media may restrict solute movement to a smaller number of available surfaces, while different types of sorption surfaces (phyllosilicate clay, iron oxides, etc.) may "dehydrate" and

become unavailable for sorption at different water contents. Also, while iron oxides and kaolinite (1:1 layer phyllosilicate mineral) may exhibit positively charged surfaces that attract anions under conditions of low pH, 2:1 phyllosilicate mineral surfaces remain largely negatively charged at low pH due to isomorphic substitution and may produce anion exclusion (Hanes, 1971). Therefore, the treatment and disruption of mineral surfaces in packed columns may affect the competing processes of adsorption and anion exclusion.

Tracers

Tracers such as bromide (Br^-), chloride (Cl^-) and fluorobenzoates (FBA), are still used as nonreactive tracers in laboratory experiments (Stollenwerk and Grove, 1985; Gamerdinger and Kaplan, 2000; Porro et al., 2000; Al-Jabri et al., 2002) despite evidence of significant anion retardation in oxide-rich materials (McMahon and Thomas, 1974; Boggs and Adams, 1992; Seaman et al., 1995; Seaman et al., 1996; Seaman, 1998) and anion exclusion in other materials (Corey et al., 1963; Thomas and Swoboda, 1970; James and Rubin, 1986; Wierenga and van Genuchten, 1989; Goncalves et al., 2001). Therefore, incorrect assumption of conservative tracer behavior ($R = 1$), can lead to misinterpretation of chemical sorption of the solute to the matrix and can have a significant impact on calculated hydrodynamic dispersion.

Anion exclusion has been found to increase with decreasing water content (James and Rubin, 1986). Biggar and Nielsen (1962) observed an early breakthrough in tritium and chloride curves with a decrease in water content from saturation to 98% of saturation. A competing process to the increase in anion

exclusion with decreasing water content would be the effect of the increased ionic strength of the soil solution as the soil becomes desaturated. This decreases the size of the double layer and acts to decrease the anion exclusion effects (de Haan, 1965; Thomas and Swoboda, 1970). James and Rubin (1986) found that the ratio of the solute velocity over the average water velocity increased significantly with lower water contents. The anion velocity has been shown to be 37% (Thomas and Swoboda, 1970) and 45% (McMahon and Thomas, 1974) greater than the average water velocity under saturated conditions. If the findings of James and Rubin (1986) apply to these studies, the average anion velocity in the unsaturated soil could be twice that of the average water velocity. Therefore, basing arrival time of anionic pollutants on average water velocity could be off by a factor of two.

Tritium (^3H) has been used as a conservative tracer to simulate water movement in soils (Biggar and Nielsen, 1962; Corey et al., 1963; James and Rubin, 1986; Boggs and Adams, 1992; Grindrod et al., 1996) since, when present in soil solution, it is primarily incorporated in water molecules, being negligibly affected by chemical interactions with the soil (Corey and Horton, 1968). However, tritium BTCs have been shown to exhibit tailing and retardation in sandstone, attributed to exchange with crystal-lattice hydroxyls (Wierenga et al., 1975).

Chromium exists in two oxidation states in soils: III (Cr^{3+} cation and CrO_2^- anion) and VI ($\text{Cr}_2\text{O}_7^{2-}$ and CrO_4^{2-}) (Bartlett and Kimble, 1976), with the trivalent oxidation state being the more stable form. Of the two forms in nature, the

trivalent form is relatively benign and the hexavalent form is relatively toxic. The Environmental Protection Agency's Maximum Contamination Level for chromium in drinking water is 0.1 ppm, but does not distinguish between oxidation states.

Chromate [Cr(VI)] is adsorbed by soil colloids such as Fe and Al oxides (Davis and Leckie, 1980; Honeyman, 1984; Zachara et al., 1987), kaolinite (Griffin et al., 1977; Zachara et al., 1988), and to a lesser extent montmorillonite (Honeyman, 1984; Rai et al., 1988) via surface complexation. Chromate sorption increases with decreasing pH and is affected by anion competition with species such as sulfate, dissolved inorganic C (Leckie et al., 1980; Zachara et al., 1987; Zachara et al., 1988), and phosphate (Aide and Cummings, 1997). Maximum adsorption by boehmite, an Al-oxyhydroxide mineral, was observed at pH 4.5 in batch equilibration studies (Aide and Cummings, 1997).

Modeling

Modeling transport processes is essential to estimating the risk or fate of solutes present in the vadose zone. However, a major limiting factor in modeling these contaminants is estimating physical and chemical parameters controlling transport processes occurring under variably-saturated conditions (Kool et al., 1987). Goncalves et al. (2001) developed pedotransfer functions (PTFs) which predict solute transport parameters from basic soil data, including particle size, bulk density, pH, porosity, water retention parameters, and saturated hydraulic conductivity. They found that predictions of R and $\log \omega$ were not descriptive ($r^2 = 0.57$ and 0.49), while more accurate predictions were reached for $\log D$ and β

values ($r^2 = 0.94$ and 0.87), using water retention and saturated hydraulic conductivity data.

Selection of the proper model is important when estimating transport parameters if the derived terms are assumed to have any mechanistic significance. If a model which assumes physical equilibrium (all water is mobile) is used to describe solute transport through a porous medium with stagnant water, the model simulation will yield small P and R values that will indicate a negative adsorption (Nkedi-Kizza et al., 1983).

Fesch et al. (1998) used CXTFIT to examine the effects of moisture content on sorbing organic compounds and found that a well parameterized transport model calibrated under saturated conditions was able to describe rate-limited advective-dispersive transport of reactive solutes under unsaturated steady-state conditions. Further information on successful predictions of solute movement would prove beneficial in the area of risk assessment.

Agreement between model simulations and experimental data is generally taken as a verification of the conceptual processes which form the basis of the model (Nkedi-Kizza et al., 1983).

Objectives

The objectives of this research are to (1) evaluate the impact of water content on the hydrodynamic dispersion of a conservative tracer (tritium) under unsaturated conditions using both the Wierenga and UFA column systems; (2) evaluate the impact of water content on the sorption properties of tracers such as bromide that are often assumed to be conservative; (3) evaluate the impact of

water content on the sorption properties of Cr(VI) under various water contents;

(4) implement modeling approaches to simulate various unsaturated solute

transport processes occurring during the study.

Notation

C	solute concentration ($M \cdot L^{-3}$)
D	hydrodynamic dispersion coefficient ($L^2 \cdot T^{-1}$)
D_e	effective molecular diffusion coefficient ($L^2 \cdot T^{-1}$)
D_o	effective molecular diffusion coefficient in free solution ($L^2 \cdot T^{-1}$)
J_w	volumetric water flux ($L \cdot T^{-1}$)
K_d	distribution coefficient ($L^3 \cdot M^{-1}$)
L	column length (L)
L_e	actual flow path length through soil column (L)
m	mass of the sorbent (M)
P	Peclet number
R	retardation factor
S	degree of saturation
S_e	effective saturation
S_r	residual saturation
t	time (T)
v	average pore water velocity ($L \cdot T^{-1}$)
V_{gw}	velocity of carrier fluid ($L \cdot T^{-1}$)
V_{sp}	velocity of solute ($L \cdot T^{-1}$)
x	mass of the adsorbed solute (M)
z	distance (L)
β	fraction of mobile water
γ	exchange rate coefficient (T^{-1})
λ	dispersivity (L)
θ	volumetric water content ($L^3 \cdot L^{-3}$)
θ_m	mobile volumetric water content ($L^3 \cdot L^{-3}$)
ρ_b	dry bulk density ($M \cdot L^{-3}$)
τ	tortuosity factor
τ_s	tortuosity factor for unsaturated media
$\tau_{s=1}$	tortuosity factor for saturated media
ω	mass transfer coefficient

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CHAPTER 2
SOLUTE TRANSPORT IN VARIABLY-SATURATED SOIL¹

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Abstract

Most subsurface contamination passes through the unsaturated zone before reaching an aquifer; however, many transport studies are conducted under saturated conditions that may not approximate the natural system under which solute transport occurs. Chromate migration was measured in sediment obtained from the Savannah River Site, SC under different water contents using vacuum and centrifuge techniques to obtain a steady-state flow regime. Leaching solutions contained 0.5 or 1.0 mM Cr(VI) and tritium in artificial groundwater. Breakthrough curves were modeled using CXTFIT assuming equilibrium conditions. Evaluation of data using a "two-region" physical nonequilibrium model confirmed that immobile water either did not exist or did not play a significant role in solute transport. Dispersion increased with decreasing water content and increasing pore water velocity, yielding a constant dispersivity value of 4.2 cm. Retardation generally increased with decreasing water content with no trend evident when K_d was calculated from R . The average K_d of all Cr(VI) experiments was $0.551 \text{ mL}\cdot\text{g}^{-1}$, very similar to the K_d derived from batch equilibration ($0.599 \text{ mL}\cdot\text{g}^{-1}$). Though results in both systems were similar, experiment duration in the vacuum system was 4 to 23 times longer than in the centrifuge system at comparable water contents. The centrifuge system also allowed column experiments to be run under a lower range in water content than the vacuum column system.

Introduction

Studies evaluating solute transport processes in the vadose zone yield important information for situations in which chemical wastes are disposed of at or below the land surface. Despite the fact that virtually all subsurface contamination must pass through the unsaturated vadose zone before reaching an aquifer, most studies have only focused on processes occurring under saturated conditions (Thomas and Swoboda, 1970; Passioura and Rose, 1971; Nkedi-Kizza et al., 1983; Powers et al., 1992; Jardine et al., 1999). This is partially due to the difficulty in maintaining steady-state flow and moisture content necessary for the controlled study of solute transport processes under unsaturated conditions. Furthermore, measurements made under saturated conditions may not approximate the conditions of the natural system under which solute transport occurs.

Differences in moisture content can influence solute migration by limiting diffusion and decreasing the pore space available for transport. Due to capillary forces, small pores tend to remain water-filled, while larger pores drain readily. In addition, the degree of saturation may also influence the availability of surface sites for reaction with the transported solutes, influencing the apparent retardation. Therefore, it is important to study the effect of moisture content on various parameters used in predicting solute transport.

The convective-dispersive equation (CDE) (Lapidus and Amundson, 1952) was developed in an attempt to quantify solute transport processes for one-

dimensional steady-state flow in a homogeneous soil column and is described by the equation:

$$R \frac{\partial C}{\partial t} = D \frac{\partial^2 C}{\partial z^2} - v \frac{\partial C}{\partial z} \quad [1]$$

where R is the retardation factor, C is the concentration ($M \cdot L^{-3}$), D is the hydrodynamic dispersion coefficient ($L^2 \cdot T^{-1}$), v is the average pore-water velocity ($L \cdot T^{-1}$), z is distance, and t is time. In order for this equation to be valid, it is assumed that all the fluid participates in the transport process.

Table 1 includes a summary of some studies evaluating the effect of moisture content on various transport parameters. Table 2 lists transport parameters, the equations pertaining to their use, and the documented changes of that parameter with decreasing water content. These tables will be used to discuss the effect of water content on solute transport parameters.

Dispersion has been shown to increase (Biggar and Nielsen, 1976; Yule and Gardner, 1978; De Smedt and Wierenga, 1979, 1984; De Smedt et al., 1986; Maraqa et al., 1997; Fesch et al., 1998; Jin et al., 2000; Gamedainger and Kaplan, 2001) or decrease (James and Rubin, 1986) with a decrease in water content. An increase in dispersion with decreasing water content may be attributed to the existence of physical nonequilibrium including regions of immobile water, a wider variation in pore water velocities when the media is desaturated, or the increase in air-filled pore space that increases the tortuosity of the solute flow path with desaturation. James and Rubin (1986) comment that a decrease in dispersion with decreasing water content could be the result of a

reduction in the possible range in pore sizes as the larger pores are emptied, though this explanation may be more applicable to structured soils.

The retardation factor is often estimated from the distribution coefficient, K_d , derived from batch equilibration studies through the equation:

$$R = 1 + \rho_b \frac{K_d}{\theta} \quad [2]$$

where ρ_b is the dry bulk density and θ is the water content. However, retardation coefficients observed in column studies often differ from values determined by batch equilibration.

Lindenmeier et al. (1995) found that K_d for U and Sr decreased 70% and 54%, respectively, with a decrease in saturation from 100% to about 28%. Uranium sorption has been shown to decrease with decreasing water content, due to rate-limited adsorption effects and faster local flow velocities present through the mobile water regions (Gamerding et al., 2001a; Gamerding et al., 2001b). Fesch et al. (1998) found an increase in retardation with decreasing water content for an organic reactive tracer. They attributed this increase to the relative increase of the number of sorption sites per water volume as the media is desaturated.

An alternative to conventional saturated systems is the use of columns under vacuum to maintain unsaturated steady-state flow, as in the commonly used Wierenga system. Disadvantages to using this technique include the inability to achieve a constant moisture content below 50% of saturation, the long experimental time required for a single breakthrough experiment, and the potential for mechanical failure due to laboratory power outages during the

extended testing period (Lindenmeier et al., 1995). Centrifuge techniques have also been employed to simulate unsaturated flow through geologic materials and have the advantage of being able to obtain stable, low water contents in a relatively short period of time, even for fine-grained materials (Gamerding and Kaplan, 2000). The centrifuge technique, initially described by Nimmo et al. (1987; 1992), was modified by researchers at Pacific Northwest National Laboratory (PNNL) (Conca and Wright, 1992) and is referred to as an unsaturated flow apparatus or UFA[®] (model L8-UFA, Beckman Coulter, Inc., Fullerton, CA). Possible problems and disadvantages associated with centrifuge column methods are maintaining stable water content along the length of the column, physical compaction problems due to the centrifugal forces, and disruption of solute transport processes during necessary stoppage for collection of effluent samples. Limited data have been obtained comparing unsaturated transport in these two systems (Lindenmeier et al., 1995). Since few studies have focused on unsaturated solute transport, the Wierenga and the UFA systems present an opportunity to examine transport parameters over a range of moisture contents.

The objectives of this research are to (1) evaluate the impact of water content on the hydrodynamic dispersion of a conservative tracer (tritium) under unsaturated conditions using both the Wierenga and UFA column systems; (2) evaluate the impact of water content on the sorption properties of tracers such as bromide that are often assumed to be conservative; (3) evaluate the impact of water content on the sorption properties of Cr(VI) under various water contents;

and (4) implement modeling approaches to simulate various unsaturated solute transport processes occurring during the study.

Materials and methods

Soil

A coarse-textured, vadose zone sediment from the Tobacco Road Formation, displaying characteristics typical of the Atlantic Coastal Plain, was collected on the Department of Energy's Savannah River Site (SRS) near Aiken, SC (Table 3). Samples were air-dried and sieved prior to batch or column experiments. Percent moisture of the air-dried soil was found to be less than 1%. The material is coarse in texture (92.65 % sand) and has a citrate-dithionite-bicarbonate extractable Fe content of 0.37g per 100g soil. Kaolinite was found to be the primary clay mineral, but the clay fraction also contains quartz, goethite, and trace amounts of mica, gibbsite, and hematite. The relationship between hydraulic conductivity (K) and water content was determined for this soil using the UFA (Figure 1).

Solutions

All solutions for adsorption isotherms and column studies were prepared in artificial groundwater (AGW). The AGW, based on routine groundwater monitoring data from the SRS (Strom and Kaback, 1992), contains the following ($\text{mg}\cdot\text{L}^{-1}$): 1.00 Ca^{2+} , 0.37 Mg^{2+} , 0.21 K^{+} , 1.40 Na^{+} , and 0.73 SO_4^{2-} . The conservative tracer for all column studies was tritium, which has a diffusion coefficient in water of $1.97 \times 10^{-9} \text{ m}^2\cdot\text{s}^{-1}$ at 20 °C (Mills and Harris, 1976).

Bromide and chromate solutions were prepared from reagent-grade KBr and $K_2Cr_2O_7$, respectively.

Batch sorption isotherm

Batch sorption isotherms were completed prior to column experiments to determine the range over which Cr(VI) sorption was linear. Chromate concentrations were then selected such that sorption was relatively moderate. This would ensure that sorption would be high enough to be easily measured, but not too high as to significantly lengthen experiment duration.

Five-gram samples of the soil were weighed into centrifuge tubes with four treatment replicates. Thirty milliliters of solution containing 0.0, 0.5, 1.0, 1.5, or 2.0 mM of Cr(VI) was added to the appropriate tubes. As a control, equivalent treatment levels were replicated in soil-free tubes to account for Cr(VI) losses in the absence of the sorbing matrix. The tubes were placed on a reciprocating shaker for about 18 hours. Previous studies on similar sediment materials indicated that this was sufficient time to achieve equilibrium with respect to Cr(VI) sorption (Seaman et al., 1999).

After equilibration, the tubes were centrifuged for 15 minutes at 15,000 rpm. The supernatant was then passed through a 0.22 μm pore size nylon membrane filter prior to Cr(VI) analysis using the diphenylcarbazide method with a detection limit of about 0.1 μM (Clesceri et al., 1989). Cr(VI) concentrations were compared to the soil-free equivalents to determine sorption based on their difference.

Vacuum-based transport experiments

The vacuum-based column experiments were conducted following the procedure of van Genuchten and Wierenga (1986). The equipment was obtained from Soil Measurement Systems, Tucson, AZ (Figure 2). The system consists of a peristaltic pump, soil column, vacuum chamber, vacuum pump and regulator, and a fraction collector. The columns were constructed of plexiglass (30.5 cm in length by 2.5 cm internal diameter) with bolted flanges at both ends and two tensiometers installed approximately 2.5 cm from each end. For unsaturated experiments, the lower end of the column had a 1.2 μm pore size membrane filter with a bubbling pressure of greater than 600 millibars while the upper end of the column had a 30 μm pore size nylon membrane filter with a bubbling pressure of 30 millibars. For saturated experiments, 30 μm pore size nylon membrane filters were used at both ends of the column.

Columns were packed with air-dried soil by adding the soil in approximately 1-cm increments and tapping lightly. Care was taken to avoid obvious layering of the material or segregation of the soil by particle size. The bulk density was calculated as the weight of solid added to the column per unit of column volume.

Columns were saturated slowly from bottom to top with AGW at flow rates less than or equal to $0.065 \text{ mL}\cdot\text{min}^{-1}$ to minimize the amount of entrapped air in the column. Water content at saturation was determined by weighing the column before and after saturation. Once saturated, the inlet solution line was switched to the top of the column and the bottom of the column was secured to the

vacuum chamber. Artificial groundwater flow was established at the top of the column with vacuum applied to the bottom of the column through the vacuum chamber. The vacuum was then adjusted with a pressure regulator until a desired stable water content was achieved. Column tensiometer data were erratic and could be not used to indicate uniform water potential. Therefore, water content was determined by weighing the column once an equilibrium value was reached. The column was also weighed at the end of the experiment to ensure the persistence of a stable water content. Since tensiometer data could not be used to ensure stable water distributions along the length of the columns, two columns were segmented and sampled for water content at the conclusion of the experiment. The flow rates and vacuum settings necessary to achieve a given degree of saturation are summarized in Appendix A.

Once a uniform moisture distribution was achieved, the inlet solution was switched to the tracer solution containing tritium and either chromate [Cr(VI)] or bromide (Br). The two concentrations used for Cr(VI) were: 0.5 and 1.0 mM, while 1.0 mM KBr was used for all bromide experiments. When possible, the inlet solution was displaced through the column until the tracer concentration in the effluent was equivalent to that in the inlet solution. At this point, the inlet solution was switched back to AGW and leached through the column until effluent tracer levels eventually fell below the detection limit.

One repacked column was used for all three unsaturated Br experiments, but fresh sediment materials were used for each Cr(VI) experiment due to the difficulty in leaching all the Cr(VI) from the column. This would have complicated

calculations of mass balance and sorption. Eight unsaturated Cr(VI) columns were run on the vacuum column apparatus.

Centrifugation-based transport experiments

The second unsaturated column system employed was the Unsaturated Flow Apparatus (UFA). The system is depicted in Figure 3 and described in detail elsewhere (Conca and Wright, 1990; Khaleel et al., 1995; Gamerdinger and Kaplan, 2000). It consists of two volumetric infusion pumps, AVI 200A (3M, St. Paul, MN) and a Beckman J6-MI centrifuge (UFA Ventures, Richland, WA), including a rotating seal, rotor, and column assembly. The column assembly consists of the sample holder and the effluent collection cup. The packed soil columns, contained in the sample holder, are 5 cm in length with an internal diameter of 3.3 cm. The effluent cup is placed at the end of the sample holder and serves to collect the column effluent during centrifugation. The rotating seal is at the center of the rotor and is the conduit through which fluid is delivered by the infusion pumps to the columns by two separate flow paths. This enables two columns to be run simultaneously with different solutions. The column assembly is connected to the rotor at the outflow of the rotating seal by a centrifuge bucket that screws in place.

Columns were packed with air-dried soil, weighed, and saturated with AGW before being placed in the centrifuge. Experiments were initiated when steady-state water contents were reached based on the gravimetric analysis of the packed columns. Columns had to be removed from the UFA for weighing

and sampling purposes. The flow rates and centrifuge speeds necessary to achieve a given degree of saturation are summarized in Appendix B.

Inlet solutions were then switched to those containing tritium and Cr(VI). Two concentrations of Cr(VI) were used: 0.5 and 1.0 mM. Flow was not initiated until the centrifuge speed reached 75% of its maximum value. This ensured that steady-state flow conditions were maintained. Whenever possible, the inlet solution was displaced through the column until the effluent concentration equaled that of the inlet solution. The column was then flushed with AGW by switching solutions again until effluent tracer levels eventually fell below the detection limit.

Since the effluent collection cups can only hold a certain volume, centrifugation must be interrupted for sample collection at specified intervals. According to Gammerdinger and Kaplan (2000), stopping centrifugation and flow for sampling purposes does not adversely affect solute breakthrough or calculation of retardation coefficients. They also reported water content as stable throughout the soil column except for the final segment at the outflow, in contact with the water-absorbent filter paper.

A new repacked soil column was used for each Cr(VI) experiment. Twenty-two columns were run in the UFA [12 columns with 0.5 mM Cr(VI) and 10 with 1.0 mM Cr(VI)], four of which were replicates. No experiments were done in the UFA using Br.

Residual Cr(VI) Extraction

Soil from the Cr(VI) columns was removed upon completion of each experiment and allowed to air dry. The soil was then thoroughly mixed and subsampled for extraction of residual Cr(VI). Five-gram samples of soil were added to centrifuge tubes with fifteen milliliters of 10 mM KH_2PO_4 extracting solution, adjusted to pH 10 (Bartlett and James, 1996). The samples were then placed on a reciprocating shaker for 24 hours and centrifuged at 15,000 rpm for 15 minutes. The supernatant was filtered with a 0.22 μm pore size nylon filter prior to analysis of Cr(VI) using the diphenylcarbazide method, as cited above. This was done for Cr(VI) columns from the vacuum and UFA experiments.

Saturated columns

Saturated experiments were conducted for both vacuum and UFA systems, using columns of the same dimensions as those used for the unsaturated experiments. These columns were saturated from bottom to top with AGW to remove as much as the air from the column as possible. The columns were then positioned horizontally, and the AGW replaced with the appropriate tracer solution. For the saturated vacuum columns, measurements of pH and EC were recorded as effluent emerged from the column.

Analysis of samples

Tritium analysis was performed by liquid scintillation counting using a Packard 2550 TR/AB Liquid Scintillation Analyzer. Two milliliters of sample were added to ten milliliters of scintillation cocktail (Packard Ultima Gold) and counted for twenty minutes. Bromide concentrations were measured using an Accumet

bromide combination ion selective electrode. All samples were brought to a high electrical conductivity before reading to ensure effects due to differences in salt content would not interfere with measurements. Chromate concentrations were determined using the diphenylcarbazide method and read using a Cary 500 Scan UV-Vis NIR Spectrophotometer at a wavelength of 540 nm.

Modeling

Tritium, Br, and Cr(VI) BTCs were modeled using CXTFIT, a program in the Stanmod software package (Version 2.0), developed by researchers at the USDA Salinity Lab, Riverside, CA. This program is an extension and update of earlier versions published over the years by van Genuchten (1979; 1981), Parker and van Genuchten (1984), and Toride et al. (1995). Tritium curves did not exhibit tailing to indicate the presence of regions of immobile water. However, both the deterministic equilibrium and two-region physical nonequilibrium convective-dispersive equation models were run on the data to ensure that regions of immobile water present in the columns did not play a significant role in solute transport. The models were run in the inverse mode for parameter estimation.

For the equilibrium model, tritium curves were used to fit values of v and D . Since tritium was run simultaneously with both Cr(VI) and Br, these v and D values were then used to find appropriate R values for Cr(VI) and Br. When using the nonequilibrium model, values of v , D , β , and ω were fit to tritium data and these values were then used to determine R for Cr(VI) and Br data.

Results and discussion

Batch sorption isotherm

The results of the Cr(VI) sorption isotherm are shown in Figure 4. The soil-free controls indicated that there was no loss of Cr(VI) to the container, and the Cr(VI)-free controls indicated that the equilibrating solution was the only source of Cr(VI). The amount of Cr(VI) sorbed was calculated as the difference between the initial Cr(VI) concentration and the final equilibrium concentration. The data are plotted as the amount of Cr(VI) sorbed vs. the equilibrium Cr(VI) concentration. The isotherm appears fairly linear up to about $75 \mu\text{g}\cdot\text{mL}^{-1}$ Cr(VI) equilibrium concentration, where it begins to level off. A graph of the change in pH with equilibrium Cr(VI) concentration is included in Appendix C. Many follow-up studies were done to examine the sorption of Cr(VI) past the range of this isotherm. However, there were difficulties in obtaining reproducible data. With increasing equilibrium concentrations, the amount sorbed to the media was often smaller than the variability between replicates. However, the isotherm shown in Figure 4 encompasses the solute concentration range used in the column studies [0.5 mM ($26 \mu\text{g}\cdot\text{mL}^{-1}$) and 1.0 mM ($52 \mu\text{g}\cdot\text{mL}^{-1}$) Cr(VI)]. Taking data over the linear range and forcing a regression line through zero produces a K_d for this isotherm of $0.599 \text{ mL}\cdot\text{g}^{-1}$ ($R^2 = 0.9992$).

Br transport experiments

The Br transport experiments are summarized in Table 4. The experiments are named to include the column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation. Therefore, Br/V-1.0-

78 is a column run with 1.0 mM Br on the vacuum-based column system at a saturation level of 78%. In discussing transport experiments, the terms "vacuum" and "UFA" will refer to the vacuum-based and centrifuge-based column experiments, respectively.

The bulk density (ρ_b), flow rate (Q), and volumetric water content (θ) values in Table 4 were determined experimentally, while the average pore water velocity (v), dispersion coefficient (D), and retardation factor (R) were results from analyzing the breakthrough curve (BTC) with CXTFIT. Parameters were slightly sensitive to input values. When fitting v and D values to the tritium BTCs, initial estimates of v were calculated from experimental conditions using the water flux and volumetric water content ($v = J_w/\theta$). Initial estimates of D were then obtained by multiplying v by 2. Changes in model results were noticed when input values of v were changed more than a factor of about 4. Values of D were less sensitive as long as appropriate values of v were used and could be varied as much as a factor of 10 or more without changing the model results. When fitting R using tritium-based v and D values, it was found that input estimates of R values were less sensitive than the other parameters and could be varied by a factor of 4 to 500 before changes in the model results occurred.

The Peclet number (P) was calculated as the pore water velocity times the column length, divided by the dispersion coefficient. The dispersivity (λ) was calculated as the dispersion coefficient (D) divided by the average pore water velocity (v). The distribution coefficient (K_d) was calculated from the retardation factor, bulk density, and volumetric water content from each column experiment.

All vacuum columns for Br could not be saturated more than about 78% even though columns were saturated from bottom to top slowly overnight in an attempt to minimize air in the soil. For the saturated Br experiment, the pH of the column effluent changed from about 5.1 to 4.4 during the pulse of the Br tracer solution. The pH returned to 5.1 when the tracer solution was changed back to AGW (graph included in Appendix D).

The BTCs for the Br transport experiments are shown in Figure 5. Breakthrough appears to be delayed as the column became desaturated with the exception of the lowest water content, 63%. All experiments exceeded the maximum breakthrough of 1.0 fraction, meaning the effluent appeared more concentrated than the influent solution. This may be due to drift in the Br electrode, although proper calibration procedures were followed throughout measurement of samples.

From Table 4, it can be seen that D apparently decreases with decreasing v . However, this relationship is not significant since the slope of the regression line includes zero, though this relationship was tested as D vs. v ($p = 0.101$) and $\text{Log}(D)$ vs. v ($p = 0.065$) in an attempt to make the data more linear. The R and calculated K_d increased with decreasing saturation and is significant ($p = 0.015$ for both relationships). This could be due to the decrease in saturation or the decrease in Q and v as the media is desaturated, resulting in greater average solute residence times. In fact, the average solute residence time increased with decreasing saturation and ranged from 99 to 636 minutes.

Bromide transport parameters were also evaluated using the two-region, nonequilibrium model to evaluate the possible role of immobile water in this system, though no tailing was observed in tritium curves. Model results indicated that the equilibrium model should be used in describing the data, since the fraction of mobile water, as indicated by β , was 95% or above for all column experiments. Also, transport parameters changed little using the nonequilibrium model. The r^2 for regression of observed vs. predicted data did not change more than 1.2% from the equilibrium to the nonequilibrium model (Appendix E). A supplementary table comparing transport parameters determined using both models is included in Appendix F.

Differences in sorption parameters fit with the equilibrium model were significant, but varied little over a range of water contents. It was concluded that the retardation of Br in this system was too low to evaluate the impact of water content on sorption reactions. Therefore, these experiments were discontinued and Cr(VI) experiments were run on the vacuum and centrifuge column systems. Chromate exhibits greater sorption than Br for this media (Seaman et al., 1999), but not to the extent that the duration of the experiment would be lengthened beyond reasonable means.

Cr(VI) transport experiments

General results of the vacuum and centrifuge-based Cr(VI) experiments will be discussed first. Then, the results of the analysis of the tritium data for all experiments (vacuum and UFA) will be discussed next as it pertains to the physical transport properties of the media. Lastly, the results of the analysis of

Cr(VI) data for all experiments (vacuum and UFA) and both concentrations (0.5 and 1.0 mM) will be discussed in light of the chemical transport properties of the media.

Vacuum-based transport experiments

As with the Br experiments, 100% saturation could not be achieved using the vacuum columns even though columns were saturated from bottom to top overnight with low flow rates. The lowest water content that could be obtained using the vacuum apparatus was around 50% saturation. Experimental duration ranged from 17 to 145 hours, depending on flow rate.

A graph of the water content distribution in the vacuum column, done similar to the method of Maraqa et al. (1997), is shown as Figure 6. The water content along the length of the column varied little when calculated on a gravimetric basis. Volumetric water contents varied more and may have included error since precise measurement of sample volumes was difficult when removing and segmenting the column materials. The column was slightly drier at the lower end which was connected to the vacuum source.

Centrifuge-based transport experiments (UFA)

Flow rates in the UFA columns varied little, as different degrees of water saturation were obtained by adjusting the centrifugation speed. For one set of columns, this failed to produce the desired water content so the flow rate was reduced. Replicate columns were run simultaneously for four column experiments and produced very similar BTCs (example graph included in Appendix G).

The duration of the experiments for the centrifuge columns ranged from 3 to 12 hours and all columns were completed in one day. This is in contrast to the vacuum experiments. The longest UFA experiment is shorter than the shortest vacuum experiment. This represents an advantage of the UFA system over the vacuum system. Another difference between the UFA and vacuum system was the range of water content that was possible. Figure 7 shows the retardation factor with water content from column experiments run on the vacuum and UFA system. It is evident that the UFA has a much greater achievable range in water content than the vacuum column system.

Physical transport parameters – tritium results

Tritium was treated as a conservative tracer to evaluate the physical properties of the media. The results for the vacuum-based transport experiments are summarized in Table 5. The experiments are named by the sorbing tracer used with the tritium [Cr(VI) in this case], but the data in this section represent the equilibrium model results for the tritium BTCs only. The sorption parameters for the Cr(VI) data will be discussed in the next section.

Experiments Cr/V-0.5-75 and Cr/V-1.0-74 represent the saturated columns. From Table 5, it does not appear that there is a linear dependence of D on v , though the column with the lowest flow rate (lowest v) produced the lowest D (Cr/V-0.5-56). Calculating P does not help reveal a relationship with this data. The results of the centrifuge-based transport experiments are presented in Tables 6 and 7. Tritium data from vacuum and UFA columns are displayed together graphically.

Appendix H shows the variation in the dispersion coefficient, D , with the average pore water velocity, v . A regression line through the data and forced through zero (De Smedt et al., 1986; Maraqa et al., 1997) yields a slope of 4.227, corresponding to the dispersivity in cm. This regression line has an $r^2 = 0.237$, but is significant ($p = 0.002$). This dispersivity value of 4.227 is comparable to values for unsaturated sand of 1.354 cm (Maraqa et al., 1997) and 7.3 cm (De Smedt et al., 1986).

In order to obtain a more linear expression of the data for statistical analysis, the graph was transformed as the $\text{Log}(D)$ with v (Appendix H). This improved the r^2 to 0.368 and produced a statistically significant relationship ($p < 0.001$) described by the equation:

$$\text{Log}(D) = 1.36 \cdot v - 0.96 \quad [3]$$

Therefore, the data do not strongly support the assertion that the dispersion coefficient is linearly related to the pore water velocity by a factor known as dispersivity. The data are better described by including an exponential function. It could not be ascertained whether this was a result of curve-fitting errors, as both parameters were not directly measured from experimental conditions. The relationship does not improve when separating data by column method (vacuum vs. UFA).

The variation in dispersion as a result of changing water content was also examined using the calculated Peclet number [$P = (v \cdot L) \cdot D^{-1}$] to account for differences in column length and pore water velocity. These results are shown in

Figure 8 as the log of the Peclet number versus the volumetric water content.

The relationship is significant ($p < 0.001$) with an equation of:

$$\text{Log}(P) = 7.59(\theta) - 0.73 \quad [4]$$

A possible explanation for the nonlinear plot of D vs. v is that the dispersivity is changing with water content. The dispersivity (λ) was calculated from the dispersion coefficient ($\lambda = D \cdot v^{-1}$). Dispersivity was then plotted with water content to see if there was a change in the transport pathways of the media with desaturation (Figure 9). The relationship is significant ($p < 0.001$) when dispersivity is expressed in the log scale and the outlier (Cr/U-0.5-25) is excluded. This plot indicates that dispersivity increases with decreasing water content, though the $r^2 = 0.4731$. This is consistent with results from other research on packed soil columns that found dispersivity to increase with decreasing water content (De Smedt et al., 1986; Fesch et al., 1998). De Smedt et al. (1986) attributed this increase in dispersivity with decreasing water content to the presence of immobile water, while Fesch et al. (1998) explained that the increase in dispersivity could be the result of the increase in tortuosity of the solute flow path as the media becomes desaturated.

To account for the possibility of immobile water in these systems, the two-region physical nonequilibrium model was fit to all tritium data. In addition to D and v , β and ω parameters were included. Tables comparing parameters fit with both models are included in Appendix I-K. All the experiments for both vacuum and centrifuge column systems had mobile water contents greater than 90% except for columns Cr/V-0.5-75 ($\beta = 0.8903$), Cr/U-1.0-66 ($\beta = 0.8842$), and Cr/U-

0.5-32 ($\beta = 0.3947$). Column Cr/V-0.5-75 is a saturated column, so these results are inconsistent with the assumption that immobile water increases with decreasing saturation. The other columns with low β values represent water contents in the middle range. These results were considered anomalies and it was therefore concluded that the nonequilibrium model was not needed to describe these data since it did not play a significant role in the transport of solutes of this system. This would follow the criteria of Garminger and Kaplan (2000), who only used parameters determined with the nonequilibrium model if the mobile water fraction, β , was less than 90%. The fit of the model parameters to the solute BTCs was not significantly improved by using the nonequilibrium model as opposed to the equilibrium model, as seen by a comparison of the r^2 values (Appendix E).

Chemical transport parameters – Cr(VI) results

A main objective of this research was to determine whether a solute's sorption parameters changed with degree of water saturation. For the saturated Cr(VI) experiments, the pH of the column effluent changed from about 4.40 to 4.18 during the pulse of the Cr(VI) tracer solution. The pH leveled off at 4.55 when the tracer solution was changed back to AGW (graph included in Appendix L). Graphs of saturated and unsaturated Cr(VI) vacuum and UFA BTCs are included in Appendix M-Q (curves not all same size because experimental pulse duration varied). However, the results in solute transport differences can best be seen through the fitted parameters. Data were well-described using CXTFIT for Cr(VI) curves in most cases (see Appendix R for graphs of observed versus

predicted data). Results for parameters fit with the equilibrium model are included in Table 8 (vacuum) and Tables 9 and 10 (UFA, 0.5 and 1.0 mM).

For the vacuum data, retardation coefficients appear to increase slightly with decreasing water content for the 1.0 mM tracer level, but calculated K_d values show no similar trend. Retardation and K_d values were usually higher with the 0.5 mM Cr(VI) columns than the 1.0 mM Cr(VI) columns, possibly indicating that sorption behavior was inconsistent with the linear K_d assumption.

Figure 10 shows the retardation coefficients as a function of water content, for both the vacuum and centrifuge experiments and both concentrations of Cr(VI) solution. The distribution coefficient, K_d , determined from the batch sorption isotherm, was used to calculate the expected retardation coefficient for each experiment, using the relationship described by [2]. These values are plotted with the column data in Figure 10 and labeled "Batch K_d ". The vacuum experiments appear to have no apparent trend with water content. The UFA experiments show an increase in R with decreasing water content. For both the UFA and vacuum column experiments, the 0.5 mM Cr(VI) experiments had generally higher retardation factors as opposed to the 1.0 mM Cr(VI) experiments, again possibly indicating nonlinear sorption. The calculated R values from batch sorption isotherm K_d followed a trend similar to the UFA 0.5 mM data. At the lowest water contents, there is a significant difference in R values calculated with the UFA system. This may be due to the fact that with very low water contents, the water film present on the soil particles becomes unstable and discontinuous, causing greater differences in solute transport. This

is also evidenced in the residual plot for the UFA 1.0 mM experiments (not shown) which showed increasing variation in R with decreasing water content.

Statistical analysis revealed that the 0.5 mM and 1.0 mM UFA experiments had a significant trend ($p < 0.001$, $p = 0.024$, respectively) of increasing R with decreasing water content. The vacuum experiments did not have a significant trend with water content for either 0.5 or 1.0 mM Cr(VI) concentration levels ($p = 0.536$, 0.252 , respectively), possibly due to the smaller range in water content over which they were conducted (0.23-0.34). Also, the retardation relationships for the UFA did not vary significantly from the vacuum system measurements for both 0.5 and 1.0 mM levels. Since data for both systems are similar and the UFA experiment durations were much shorter than the vacuum experiments at comparable water contents, it would appear that residence time had no significant effect on transport parameters over this range in water content (0.07-0.41) and solute residence time (4-300 min.). It would have been expected for the 0.5 mM curve to be different from the 1.0 mM curve, but the variation in data did not allow for a significant difference between the treatment levels. The general increasing trend of R with water content in these two systems agrees with the relationship described by equation [2].

In the preceding section, a two-region nonequilibrium model was used to determine if immobile water played a significant role in solute transport. The conclusion was that immobile water would not need to be accounted for in describing solute transport in this system since nearly all the mobile water fractions were greater than 90%. However, to further indicate that this had no

effect on transport parameters, the parameters that were fit to the tritium data with the nonequilibrium model (D , v , β , and ω) were used to fit R for the Cr(VI) data. The values are not significantly different than those fit with the equilibrium model as shown in Appendix S-U. A graph is also included that shows R from the equilibrium model graphed against the R from the nonequilibrium model (Appendix V). The slope of this relationship is very close to 1. This further indicates that the data is sufficiently described using the equilibrium model and that immobile water does not play a significant role in solute transport in this system. A comparison of the r^2 values for the determination of R in both the equilibrium and nonequilibrium models is included in Appendix E.

To examine whether sorption was changing with water content, the K_d of each column experiment was calculated from the R value, using bulk density and water content measurements from each experiment. The results are shown in Figure 10. The average of all Cr(VI) column experiment K_d values (0.5 and 1.0 mM) was $0.551 \text{ mL}\cdot\text{g}^{-1}$, similar to the batch sorption isotherm K_d value of $0.599 \text{ mL}\cdot\text{g}^{-1}$ (Figure 4). Also, there was no significant difference between K_d values determined with the UFA and vacuum systems. Generally, the 0.5 mM data points are higher than those of the 1.0 mM data, indicating a possible deviation from the linear sorption. However, there does not appear to be a trend of K_d with water content. This was verified with statistical analysis. Therefore, it appears that K_d does not significantly vary with water content in this system. This differs from the results of Lindenmeier et al. (1995) who found a decrease in K_d for U and Sr with a decrease in saturation. They speculated that this could be due to

insufficient residence time or a decrease in the relative number of available sorption sites per volume of soil as the water content is decreased. However, the K_d for benzene has been shown to change little with water content when changing soil:water ratios are accounted for (Maraqa et al., 1999). Comparisons between the results of this study and those of other researchers are complicated by the fact that different sorbing tracers, soils, and equipment are used in most experiments. However, considering the limited number of studies evaluating solute transport in unsaturated media, it is important to examine these differences.

Though Lindenmeier et al. (1995) used vacuum and UFA systems similar to those used in the present study with a relatively sandy soil, their results are different than the relationships seen here. This may be due to the difference in the properties of U and Sr versus Cr(VI) as it relates to the behavior of the tracers with certain available sorption sites that may change with degree of water saturation.

To examine the idea that sorption may be a function of residence time, the conservative tracer, tritium, was used to calculate the amount of Cr(VI) that was sorbed by the soil during the course of each experiment. This was accomplished by using a mass balance calculation to find the difference between the tritium and Cr(VI) BTCs. These values are displayed in Table 11. The average value for all experiments was 22.2 mg Cr(VI) per kg soil. There is a significant ($p < 0.001$) positive linear relationship for the UFA 0.5 mM data between the amount sorbed and the volumetric water content (Figure 11). However, this trend was

not significant for the UFA 1.0 mM or the vacuum column data (not shown in graph). This would indicate that there was greater sorption when the soil was more saturated. This could point to the process of desaturation limiting the number of surfaces available for sorption. However, there was also a significant positive linear relationship between the amount sorbed and the average retention time for the 0.5 and 1.0 mM UFA data ($p = 0.007$ and 0.05 , respectively) (Figure 11). The 0.5 and 1.0 mM treatment levels were not significantly different from each other. The average retention time was calculated as the volume of water present in the soil divided by the flow rate through the soil. The flow rate was changed only once for the UFA column experiments. Therefore, with decreasing saturation, the soil water decreases, thus decreasing the average retention time of the solute in the soil. This indicates that the amount sorbed may be more a function of kinetics as opposed to decreased availability of surface sites with changing pathways of solute transport. However, this is inconsistent with earlier retardation data (Figure 10) that showed similar values for UFA and vacuum columns, though these systems have significantly different solute retention times.

Effluent recoveries of the Cr(VI) in the columns ranged from 53 to 111.4 %, mainly due to insufficient leaching with AGW to remove the sorbed fraction. Average recovery was 81.4%. Therefore, an extraction procedure with 10 mM of KH_2PO_4 was carried out on the soil after the column experiments were completed. Average recovery after the extraction was 92.5%. A table with the percent recovery values is included in Appendix W. There was no relationship between the experimental recovery and any transport parameters.

Summary and conclusions

The objective of these solute transport experiments was to evaluate the effect of water content on the dispersion and sorption properties of tritium, bromide, and chromate in a sandy sediment characteristic of the Atlantic Coastal Plain. Two methods were employed to create a steady-state unsaturated flow regime within the packed soil columns: a vacuum-based Wierenga column system and a centrifuge-based Unsaturated Flow Apparatus (UFA). It was found that there was no significant difference in solute transport parameters derived with these two systems. The experimental duration of the columns on the vacuum-based system was 4 to 23 times longer than those run with the centrifuge-based system at comparable water contents. The vacuum system was only able to reach water contents as low as 50% of saturation, while the UFA reached water contents as low as 16% of saturation. All UFA experiments could be finished in one day, though the system has to be stopped for sampling purposes at specified intervals.

Dispersion increased with pore water velocity and decreasing water content (as indicated by Peclet number). However, these relationships were not linear in nature. In general, retardation increased with decreasing water content, but this relationship was only significant for the UFA data. However, calculation of the distribution coefficient from the retardation factor indicated no trend with water content. The average of all Cr(VI) column experiment K_d values, $0.551 \text{ mL}\cdot\text{g}^{-1}$, was similar to the batch sorption isotherm K_d value of $0.599 \text{ mL}\cdot\text{g}^{-1}$.

The total amount of Cr(VI) sorbed to the soil throughout the course of the experiment increased with increasing water content. However, Cr(VI) sorption also increased with the average solute retention time within the soil. It was unclear whether Cr(VI) sorption was influenced by changing solute flow paths with desaturation, or was a factor of average solute retention time.

Notation

C	solute concentration ($M \cdot L^{-3}$)
D	hydrodynamic dispersion coefficient ($L^2 \cdot T^{-1}$)
J_w	water flux ($L \cdot T^{-1}$)
K_d	distribution coefficient ($L^3 \cdot M^{-1}$)
P	Peclet number
Q	volumetric flow rate ($L^3 \cdot T^{-1}$)
R	retardation factor
t	time (T)
v	average pore water velocity ($L \cdot T^{-1}$)
z	distance (L)
β	fraction of mobile water
λ	dispersivity (L)
θ	volumetric water content ($L^3 \cdot L^{-3}$)
ρ_b	dry bulk density ($M \cdot L^{-3}$)
ω	mass transfer coefficient

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Table 1. Review of studies evaluating changes in transport parameters with water content[†]

Investigator	Tracer	Material	Column LxD	Method	Saturation	Results
(Maraqa et al., 1999)	³ H, organic tracers	sand	30.2 x 5.45	vacuum	43-100%	<i>R</i> increases with decreasing θ Explained by s:l ratio
(Maraqa et al., 1997)	³ H	sand	30.2 x 5.45	vacuum	43-100%	<i>D</i> increases with decreasing θ and increasing <i>v</i> $\lambda_{\text{unsat}}/\lambda_{\text{sat}} \sim 1.9-2.8$
(Gamerding and Kaplan, 2001)	Br	sand tuff	6 x 4.5	centrifuge	5.4-79%	<i>D</i> increases with decreasing θ
(Gamerding et al., 2001a)	U(VI)	silt loam	6 x 4.5	centrifuge	38-90%	<i>R</i> increase with decreasing <i>v</i> <i>K_d</i> decrease with decreasing θ Stopping centrifuge increased <i>K_d</i>
(De Smedt and Wierenga, 1979, 1984; Gamerding et al., 2001a)	³⁶ Cl	glass beads	30 x 5.4	vacuum	20-100%	<i>D</i> and IM increases with decreasing θ
(De Smedt et al., 1986)	³ H	sand	100 x 15.4	unsteady flow	15-100%	λ increases with decreasing θ Explained by IM
(Porro et al., 2000)	Br Sr	basalt	25.4 x 2.77	vacuum	40-100%	<i>K_d</i> not vary with θ Column <i>K_d</i> greater than batch <i>D</i> increases with decreasing θ
(James and Rubin, 1986)	³ H	sand	48.5 x 5	vacuum	41-100%	λ decrease with decreasing θ
(Fesch et al., 1998)	Cl organic tracers	coated sand	13.5 x 5.3	vacuum	45-86%	λ , <i>R</i> increases with decreasing θ
(Corey et al., 1963)	³ H Cl	sand- stone	varied	vacuum	59-100%	<i>D</i> increases with <i>v</i> and column length <i>D</i> varies little with θ

[†]Abbreviations: *L*, length; *D*, diameter; *R*, retardation factor; θ , volumetric water content; s:l, soil:liquid; *D*, dispersion coefficient; *v*, average pore water velocity; λ , dispersivity; unsat, unsaturated; sat, saturated; *K_d*, distribution coefficient; and IM, immobile water.

Table 2. Definitions of transport parameters and the effect of the variation of water content[†]

Symbol	Name	Definition	Change with decreasing θ	References
D	Dispersion coefficient	$^{\ddagger}D = \lambda \cdot v + D_e$	increases	(De Smedt and Wierenga, 1979, 1984; Maraqa et al., 1997; Porro et al., 2000; Gamerdinger and Kaplan, 2001)
			decreases	(Jardine et al., 1993)
			little	(Corey et al., 1963)
λ	Dispersivity	$^{\natural}\lambda = \frac{D}{v}$	increases	(De Smedt et al., 1986; Fesch et al., 1998)
			decreases	(James and Rubin, 1986)
P	Peclet number	$P = \frac{v \cdot L}{D}$	decreases	(Porro et al., 2000)
R	Retardation factor	$^{\S}R = 1 + \frac{\rho_b \cdot K_d}{\theta}$	increases	(Fesch et al., 1998; Maraqa et al., 1999)
K_d	Distribution coefficient	$^{\S}K_d = \frac{\theta \cdot (R - 1)}{\rho_b}$	decreases	(Lindenmeier et al., 1995; Gamerdinger et al., 2001a)
			little	(Maraqa et al., 1999)
IM	Immobile Water	Portion of water excluded from advective transport	increases	(Gaudet et al., 1977; De Smedt and Wierenga, 1984; De Smedt et al., 1986)
			none	(Maraqa et al., 1997)

[†]Abbreviations: θ , volumetric water content; v , average pore water velocity; D_e , effective molecular diffusion coefficient; L , column length; IM, immobile water, and ρ_b , bulk density.

[‡](Bear, 1969)

[‡](Passioura, 1971; Passioura and Rose, 1971)

[§](Bouwer, 1991)

Table 3. Physical and chemical characteristics of Tobacco Road sand used in column and batch studies

[†] Texture		Sand
[‡] Particle-size distribution (wt. %)	[¶] Soil mineralogy	
Sand (>53 μm)	q>>>f	92.65
Silt (53-2 μm)	q>>>k	2.55
Clay (<2 μm)	k>>q>goe, mica, gibb, hem	4.80
[§] Water dispersible clay (wt.%)		0.056
[#] Bulk density _{disturbed} (g·cm ⁻³)		1.49
^{††} pH _{deionized water}		5.10
^{††} pH _{AGW}		5.00
^{††} pH _{KCl}		4.21
^{##} EC (μS·cm ⁻¹)		4.29
^{¶¶} Total organic carbon (g/100g)		0.09
^{§§} BET surface area (m ² ·g ⁻¹)		2.506
[■] Cation exchange capacity (meq/100g)		2.3
	⁺⁺⁺Total metals, g/100 g soil	
Fe		0.48923
Cr		0.00018
	Extractable Fe, g Fe/100 g soil	
⁺⁺⁺ CDB Fe		0.37
^{¶¶¶} AO Fe		0.02

[†]general soil classification of US Department of Agriculture

[‡]hydrometer analysis

[¶]determined by X-ray diffraction, q = quartz, f = feldspar, k = kaolinite, goe = goethite, gibb = gibbsite, hem = hematite

[§]micro-pipette method (Miller and Miller, 1987)

[#]ASTM D853-83

^{††}1:2 soil:solution in deionized water, AGW, or 1 M KCl for 30 minutes

^{††}1:2 soil: solution in deionized water

^{¶¶}TOC, total organic carbon, dry combustion method (Nelson, 1982)

^{§§}by Micrometrics ASAP-2010

^{##}ammonium acetate method (Rhoades, 1982; Thomas, 1982)

⁺⁺⁺EPA method 3051; Microwave assisted acid digestion

⁺⁺⁺citrate-dithionite-bicarbonate extraction (Jackson et al., 1986)

^{¶¶¶}ammonium oxalate extraction (Jackson et al., 1986)

Table 4. Bromide transport parameters determined by direct measurement or from solute breakthrough curves evaluated using the equilibrium and nonequilibrium transport models[†]

Experiment [‡]	ρ_b (g·cm ⁻³)	Q (cm ³ ·min ⁻¹)	θ	v (cm·min ⁻¹)	D	P	λ (cm)	R	K_d (mL·g ⁻¹)
Br/V-1.0-78	1.47	0.694	0.35	0.4173	0.2182	58.3	0.5229	1.289	0.069
Br/V-1.0-73	1.47	0.475	0.33	0.3749	0.1348	84.8	0.3596	1.347	0.078
Br/V-1.0-68	1.47	0.190	0.31	0.1558	0.0641	74.2	0.4114	1.393	0.083
Br/V-1.0-63	1.47	0.069	0.29	0.0796	0.0719	33.8	0.9033	1.595	0.109

[†]Abbreviations are as follows: ρ_b , bulk density; Q , flow rate; θ , volumetric water content; v , average pore water velocity; D , hydrodynamic dispersion coefficient; P , Peclet number; λ , dispersivity; R , retardation factor; and K_d , distribution coefficient.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

Table 5. Tritium transport parameters for vacuum system, derived by direct measurement or from solute breakthrough curves evaluated using the equilibrium transport model[†]

Experiment [‡]	ρ_b (g·cm ⁻³)	Q (cm ³ ·min ⁻¹)	θ	v (cm·min ⁻¹)	D	P	λ (cm)
Cr/V-0.5-75	1.44	0.813	0.34	0.5221	0.2331	68.3	0.4465
Cr/V-0.5-63	1.43	0.706	0.29	0.2808	1.4560	5.9	5.185
Cr/V-0.5-62	1.45	0.735	0.28	0.5317	0.5470	29.6	1.029
Cr/V-0.5-56	1.46	0.131	0.25	0.1097	0.0619	54.1	0.5643
Cr/V-0.5-54	1.45	0.700	0.24	0.4360	0.0688	193.4	0.1578
Cr/V-1.0-74	1.49	0.789	0.32	0.6318	0.2718	70.9	0.4302
Cr/V-1.0-64	1.50	0.671	0.28	0.4950	0.0940	160.6	0.1899
Cr/V-1.0-62	1.50	0.730	0.27	0.5256	0.2570	62.4	0.4890
Cr/V-1.0-58	1.48	0.664	0.25	0.5510	0.1664	101.0	0.3020
Cr/V-1.0-51	1.45	0.142	0.23	0.1198	0.1223	29.9	1.021

[†]Abbreviations are as follows: ρ_b , bulk density; Q , flow rate; θ , volumetric water content; v , average pore water velocity; D , hydrodynamic dispersion coefficient; P , Peclet number; and λ , dispersivity.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

Table 6. Tritium transport parameters for the UFA (centrifuge system), 0.5 mM Cr(VI) tracer solution, determined using the equilibrium transport model[†]

Experiment [‡]	ρ_b (g·cm ⁻³)	Q (cm ³ ·min ⁻¹)	θ	v (cm·min ⁻¹)	D	P	λ (cm)
Cr/U-0.5-75	1.47	0.792	0.34	0.2033	0.0916	67.7	0.4506
Cr/U-0.5-70	1.57	0.810	0.29	0.3622	0.2264	8.0	0.6251
Cr/U-0.5-68	1.58	0.810	0.28	0.3401	0.2276	7.5	0.6814
Cr/U-0.5-57	1.54	0.810	0.24	0.6052	0.6514	4.6	1.076
Cr/U-0.5-51	1.55	0.500	0.21	0.2345	0.3335	3.5	1.422
Cr/U-0.5-45	1.56	0.810	0.19	0.5381	0.5166	5.2	0.9600
Cr/U-0.5-32	1.53	0.810	0.13	0.6720	1.007	3.3	1.499
Cr/U-0.5-31	1.52	0.810	0.13	0.5704	1.785	1.6	3.129
Cr/U-0.5-28	1.56	0.810	0.11	0.7020	1.188	3.0	1.692
Cr/U-0.5-25	1.57	0.810	0.10	0.0066	8.411	0.0039	1274
Cr/U-0.5-19	1.53	0.810	0.08	1.2040	10.78	0.6	8.953
Cr/U-0.5-18	1.50	0.810	0.08	0.9767	16.56	0.3	16.96
Cr/U-0.5-16	1.48	0.810	0.07	0.6786	2.37	1.4	3.492

[†]Abbreviations are as follows: ρ_b , bulk density; Q , flow rate; θ , volumetric water content; v , average pore water velocity; D , hydrodynamic dispersion coefficient; P , Peclet number; and λ , dispersivity.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

Table 7. Tritium transport parameters for the UFA (centrifuge system), 1.0 mM Cr(VI) tracer solution, determined using the equilibrium transport model[†]

Experiment[‡]	ρ_b (g·cm⁻³)	Q (cm³·min⁻¹)	θ	v (cm·min⁻¹)	D	P	λ (cm)
Cr/U-1.0-94	1.47	0.792	0.41	0.1741	0.2617	20.3	1.503
Cr/U-1.0-66	1.57	0.810	0.27	0.3398	0.0101	167.9	0.0297
Cr/U-1.0-57a	1.56	0.500	0.23	0.2676	0.4168	3.2	1.556
Cr/U-1.0-57b	1.54	0.810	0.24	0.2707	0.1623	8.3	0.5996
Cr/U-1.0-44	1.58	0.810	0.18	0.5807	0.6764	4.3	1.165
Cr/U-1.0-33	1.57	0.810	0.14	0.6005	0.5527	5.4	0.9204
Cr/U-1.0-26a	1.53	0.810	0.11	0.6612	3.3190	1.0	5.020
Cr/U-1.0-26b	1.53	0.810	0.11	0.6460	3.0920	1.0	4.786
Cr/U-1.0-21	1.54	0.810	0.09	1.4420	4.282	1.7	2.969
Cr/U-1.0-19	1.49	0.810	0.08	0.6786	2.37	1.4	3.492
Cr/U-1.0-17	1.51	0.810	0.07	1.2840	5.816	1.1	4.530

[†]Abbreviations are as follows: ρ_b , bulk density; Q , flow rate; θ , volumetric water content; v , average pore water velocity; D , hydrodynamic dispersion coefficient; P , Peclet number; and λ , dispersivity.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

Table 8. Cr(VI) transport parameters for vacuum system, derived by direct measurement or from solute breakthrough curves evaluated using the equilibrium transport model[†]

Experiment[‡]	ρ_b (g·cm⁻³)	Q (cm³·min⁻¹)	θ	R	K_d (mL·g⁻¹)
Cr/V-0.5-75	1.44	0.813	0.34	4.540	0.836
Cr/V-0.5-63	1.43	0.706	0.29	3.296	0.466
Cr/V-0.5-62	1.45	0.735	0.28	4.938	0.760
Cr/V-0.5-56	1.46	0.131	0.25	6.476	0.938
Cr/V-0.5-54	1.45	0.700	0.24	4.578	0.592
Cr/V-1.0-74	1.49	0.789	0.32	3.284	0.491
Cr/V-1.0-64	1.50	0.671	0.28	3.032	0.379
Cr/V-1.0-62	1.50	0.730	0.27	3.222	0.400
Cr/V-1.0-58	1.48	0.664	0.25	3.314	0.391
Cr/V-1.0-51	1.45	0.142	0.23	4.074	0.488

[†]Abbreviations are as follows: ρ_b , bulk density; Q , flow rate; θ , volumetric water content; R , retardation coefficient; and K_d , distribution coefficient.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

Table 9. Cr(VI) transport parameters for the UFA (centrifuge system), 0.5 mM Cr(VI) tracer solution, determined using the equilibrium transport model[†]

Experiment [‡]	ρ_b (g·cm ⁻³)	Q (cm ³ ·min ⁻¹)	θ	R	K_d (mL·g ⁻¹)
Cr/U-0.5-75	1.47	0.792	0.34	3.439	0.564
Cr/U-0.5-70	1.57	0.810	0.29	5.303	0.795
Cr/U-0.5-68	1.58	0.810	0.28	5.096	0.726
Cr/U-0.5-57	1.54	0.810	0.24	8.496	1.168
Cr/U-0.5-51	1.55	0.500	0.21	6.283	0.716
Cr/U-0.5-45	1.56	0.810	0.19	7.063	0.738
Cr/U-0.5-32	1.53	0.810	0.13	8.000	0.595
Cr/U-0.5-31	1.52	0.810	0.13	8.813	0.668
Cr/U-0.5-28	1.56	0.810	0.11	8.819	0.551
Cr/U-0.5-25	1.57	0.810	0.10	9.940	0.569
Cr/U-0.5-19	1.53	0.810	0.08	15.33	0.749
Cr/U-0.5-18	1.50	0.810	0.08	12.16	0.595
Cr/U-0.5-16	1.48	0.810	0.07	2.787	0.085

[†]Abbreviations are as follows: ρ_b , bulk density; Q , flow rate; θ , volumetric water content; R , retardation coefficient; and K_d , distribution coefficient.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

Table 10. Cr(VI) transport parameters for the UFA (centrifuge system), 1.0 mM Cr(VI) tracer solution determined using the equilibrium transport model[†]

Experiment [‡]	ρ_b (g·cm ⁻³)	Q (cm ³ ·min ⁻¹)	θ	R	K_d (mL·g ⁻¹)
Cr/U-1.0-94	1.47	0.792	0.41	2.308	0.365
Cr/U-1.0-66	1.57	0.810	0.27	2.989	0.342
Cr/U-1.0-57a	1.56	0.500	0.23	4.745	0.552
Cr/U-1.0-57b	1.54	0.810	0.24	2.964	0.306
Cr/U-1.0-44	1.58	0.810	0.18	4.409	0.388
Cr/U-1.0-33	1.57	0.810	0.14	4.664	0.327
Cr/U-1.0-26a	1.53	0.810	0.11	11.46	0.752
Cr/U-1.0-26b	1.53	0.810	0.11	7.122	0.440
Cr/U-1.0-21	1.54	0.810	0.09	12.07	0.647
Cr/U-1.0-19	1.49	0.810	0.08	2.858	0.100
Cr/U-1.0-17	1.51	0.810	0.07	6.399	0.250

[†]Abbreviations are as follows: ρ_b , bulk density; Q , flow rate; θ , volumetric water content; R , retardation coefficient; and K_d , distribution coefficient.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

Table 11. Amount of Cr(VI) sorbed by soil during the course of the experiment

Experiment[†]	Sorbed Cr(VI) (mg·kg⁻¹)
Cr/V-0.5-75	33.1
Cr/V-0.5-66	21.4
Cr/V-0.5-63	26.5
Cr/V-0.5-56	32.5
Cr/V-0.5-54	26.8
Cr/V-1.0-74	28.1
Cr/V-1.0-64	27.3
Cr/V-1.0-62	21.0
Cr/V-1.0-58	21.9
Cr/V-1.0-51	32.5
Cr/U-0.5-75	22.4
Cr/U-0.5-70	28.5
Cr/U-0.5-68	27.5
Cr/U-0.5-57	24.4
Cr/U-0.5-51	20.7
Cr/U-0.5-45	21.9
Cr/U-0.5-32	15.6
Cr/U-0.5-31	13.8
Cr/U-0.5-28	23.5
Cr/U-0.5-25	14.5
Cr/U-0.5-19	10.7
Cr/U-0.5-18	11.5
Cr/U-0.5-16	13.1
Cr/U-1.0-94	25.0
Cr/U-1.0-66	23.4
Cr/U-1.0-57a	29.6
Cr/U-1.0-57b	27.7
Cr/U-1.0-44	17.3
Cr/U-1.0-33	18.0
Cr/U-1.0-26a	15.8
Cr/U-1.0-26b	13.2
Cr/U-1.0-21	19.7
Cr/U-1.0-19	26.4
Cr/U-1.0-17	18.4

[†]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

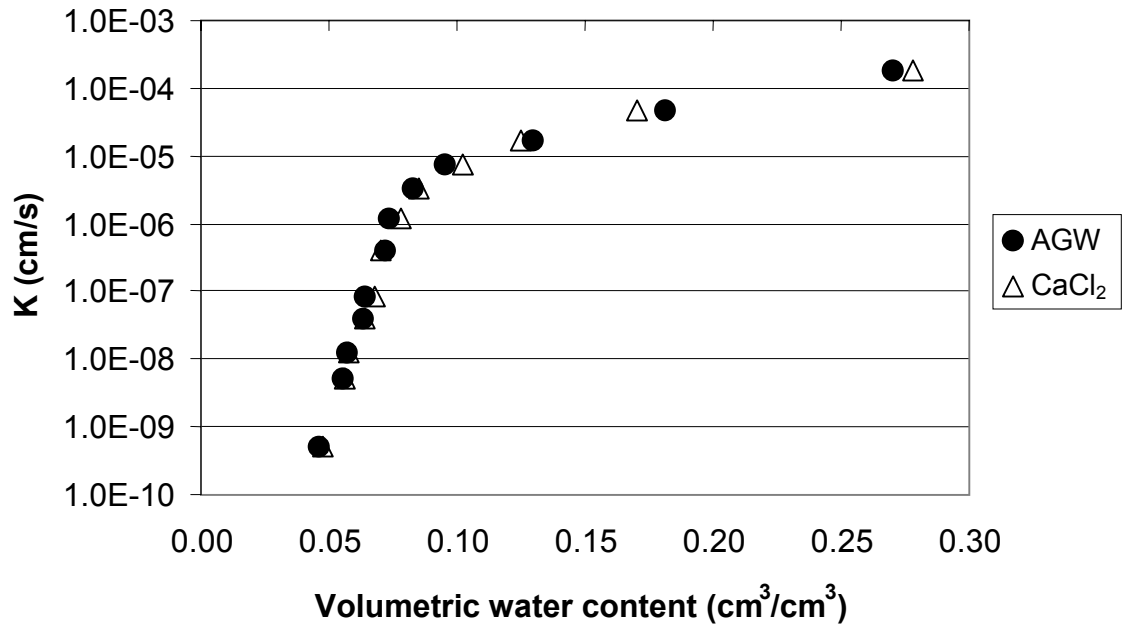


Figure 1. Unsaturated hydraulic conductivity curves for sandy loam soil obtained using the UFA with artificial groundwater (AGW) and calcium chloride leaching solutions.

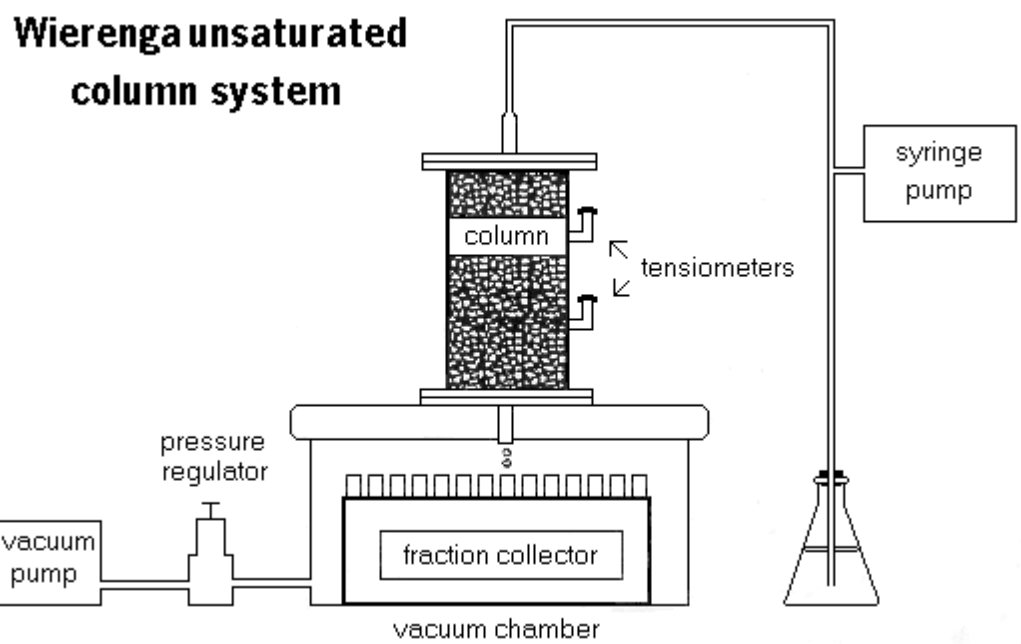


Figure 2. Diagram of the Wierenga column setup for unsaturated flow (redrawn from Jin et al., 2000).

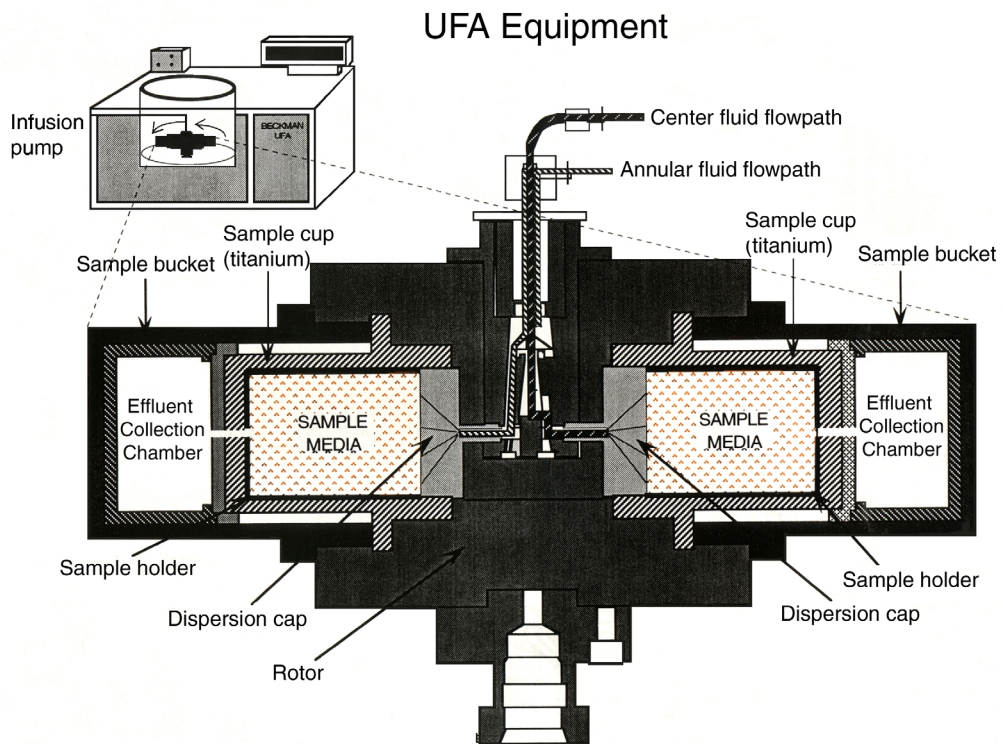


Figure 3. Diagram of the Unsaturated Flow Apparatus components (Khaleel et al., 1995).

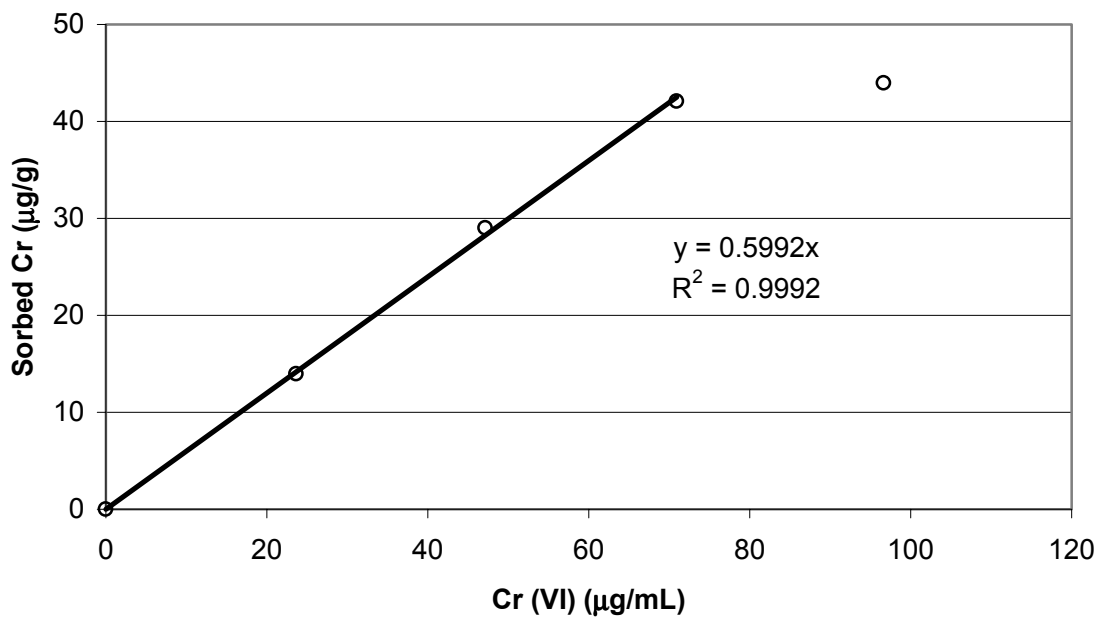


Figure 4. Cr(VI) sorption isotherm on loamy sand soil at a soil:solution ratio of 1:6 by mass, equilibrated for 24 hours.

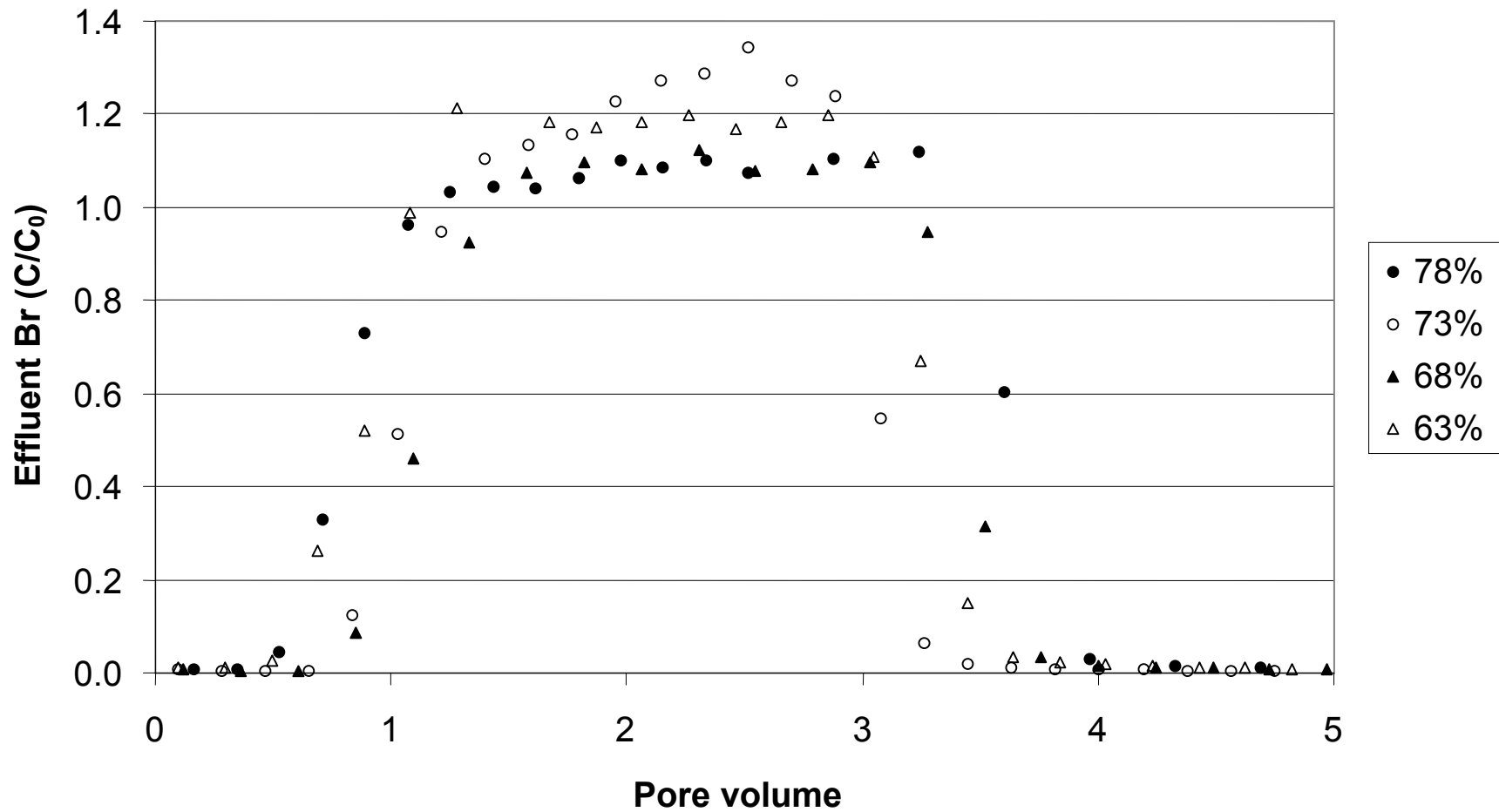


Figure 5. Bromide breakthrough curves for four levels of column saturation. C/C_0 = the effluent Br over the influent Br concentration.

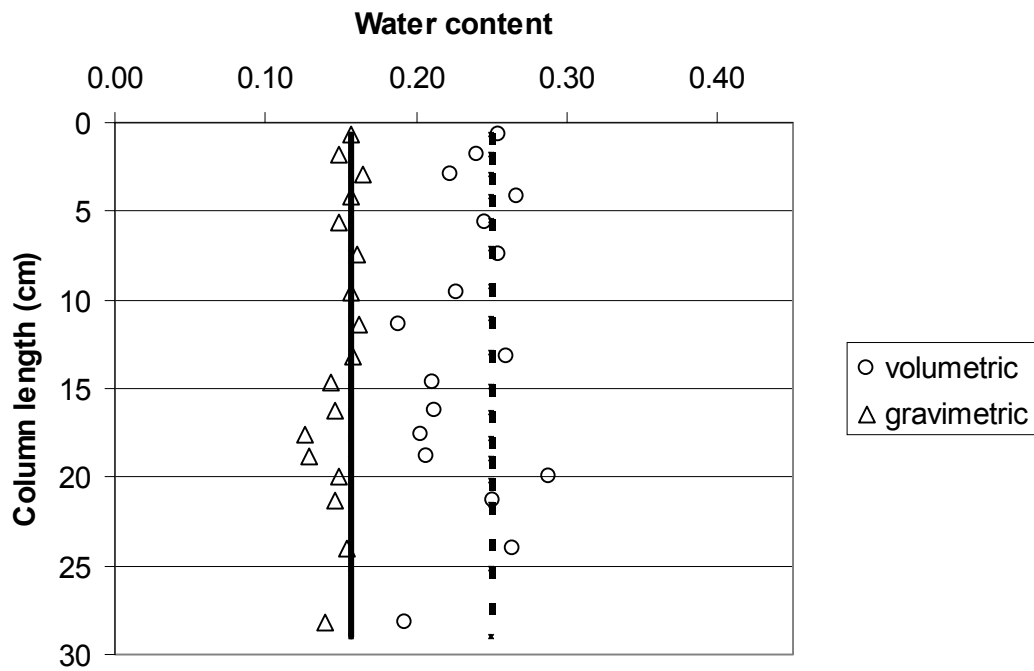


Figure 6. Volumetric and gravimetric water content with column length for vacuum column experiment under 50 cbars of vacuum pressure. Solid and dotted lines represent the assumed volumetric and gravimetric water contents based on difference in weight of entire column when dry and moist.

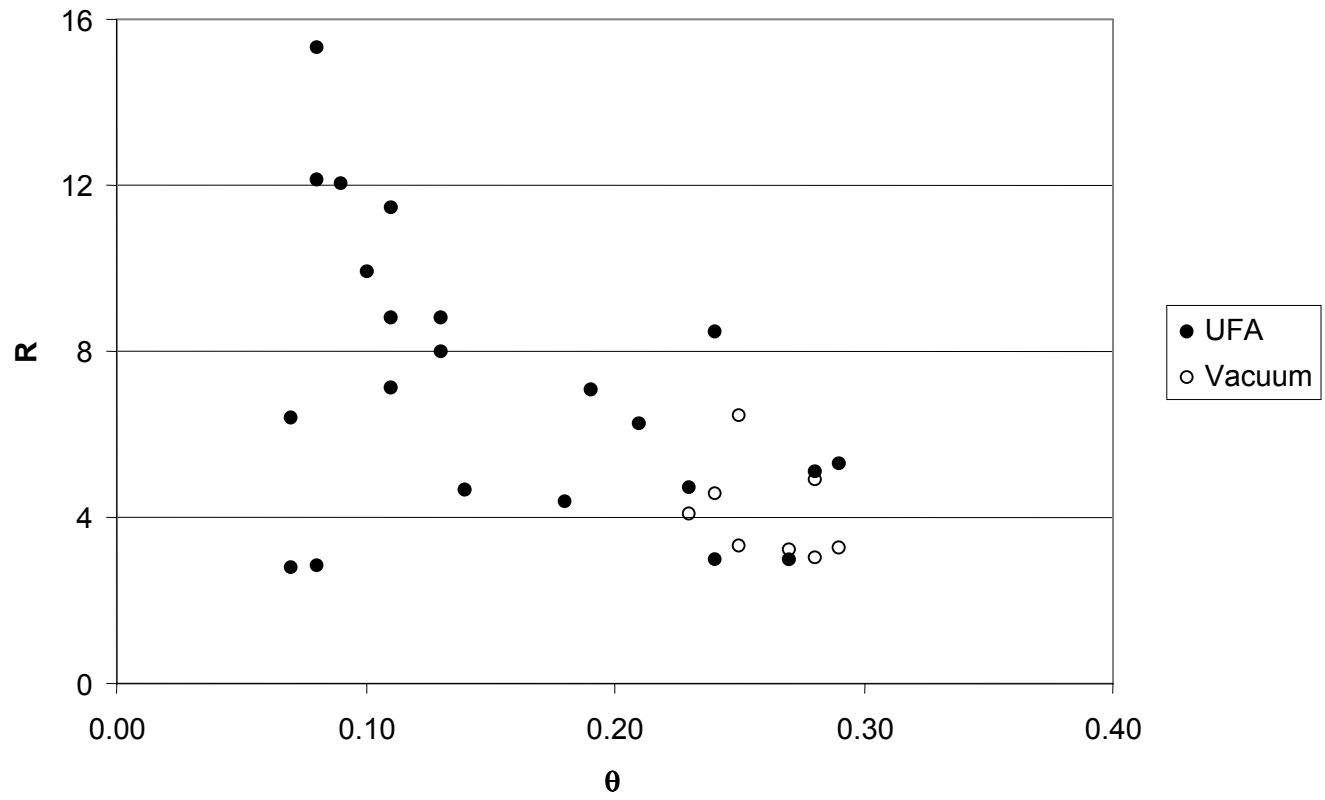


Figure 7. Retardation in unsaturated media determined using centrifuge-based (UFA) and vacuum-based column systems.

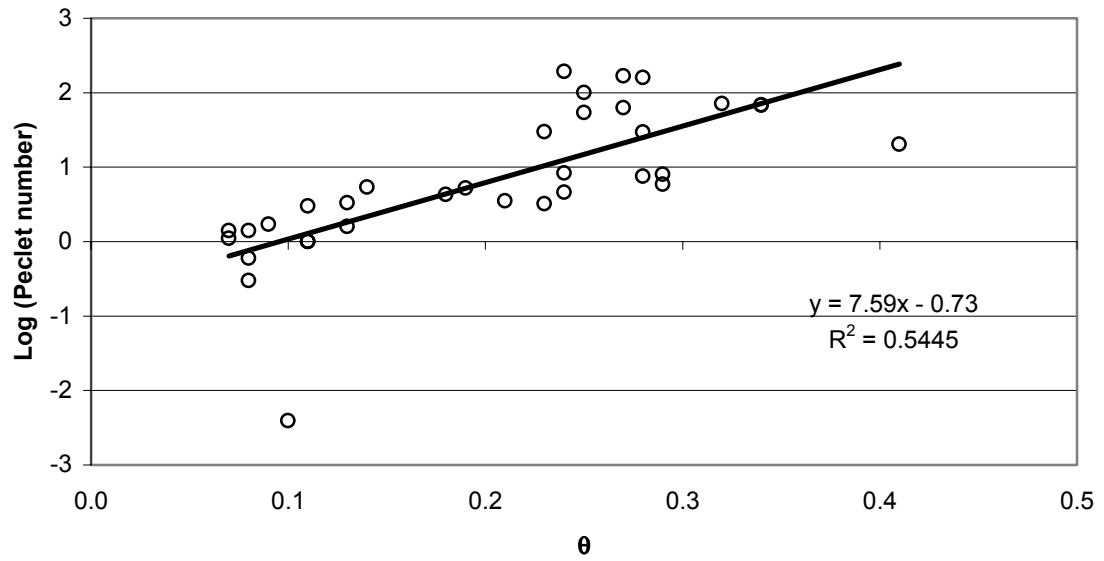


Figure 8. Log of Peclet number with volumetric water content for vacuum and UFA tritium column data.

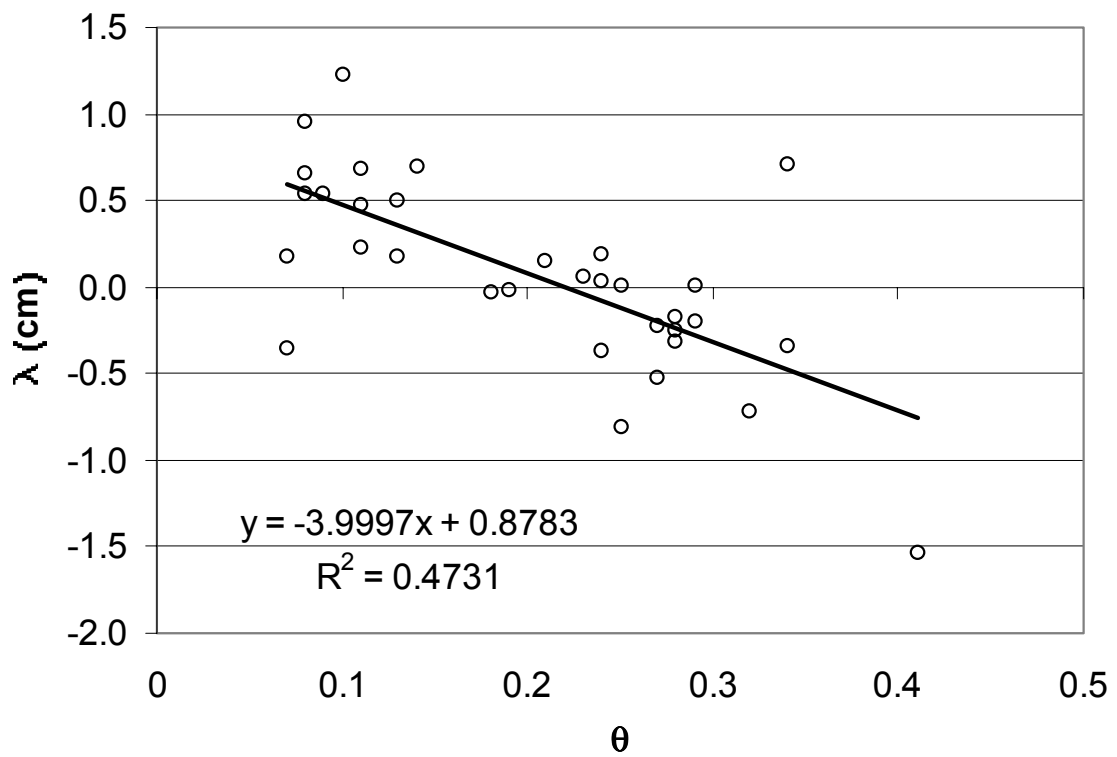


Figure 9. Log of dispersivity with water content for tritium data from vacuum and centrifuge column experiments, determined from D fit with the equilibrium model.

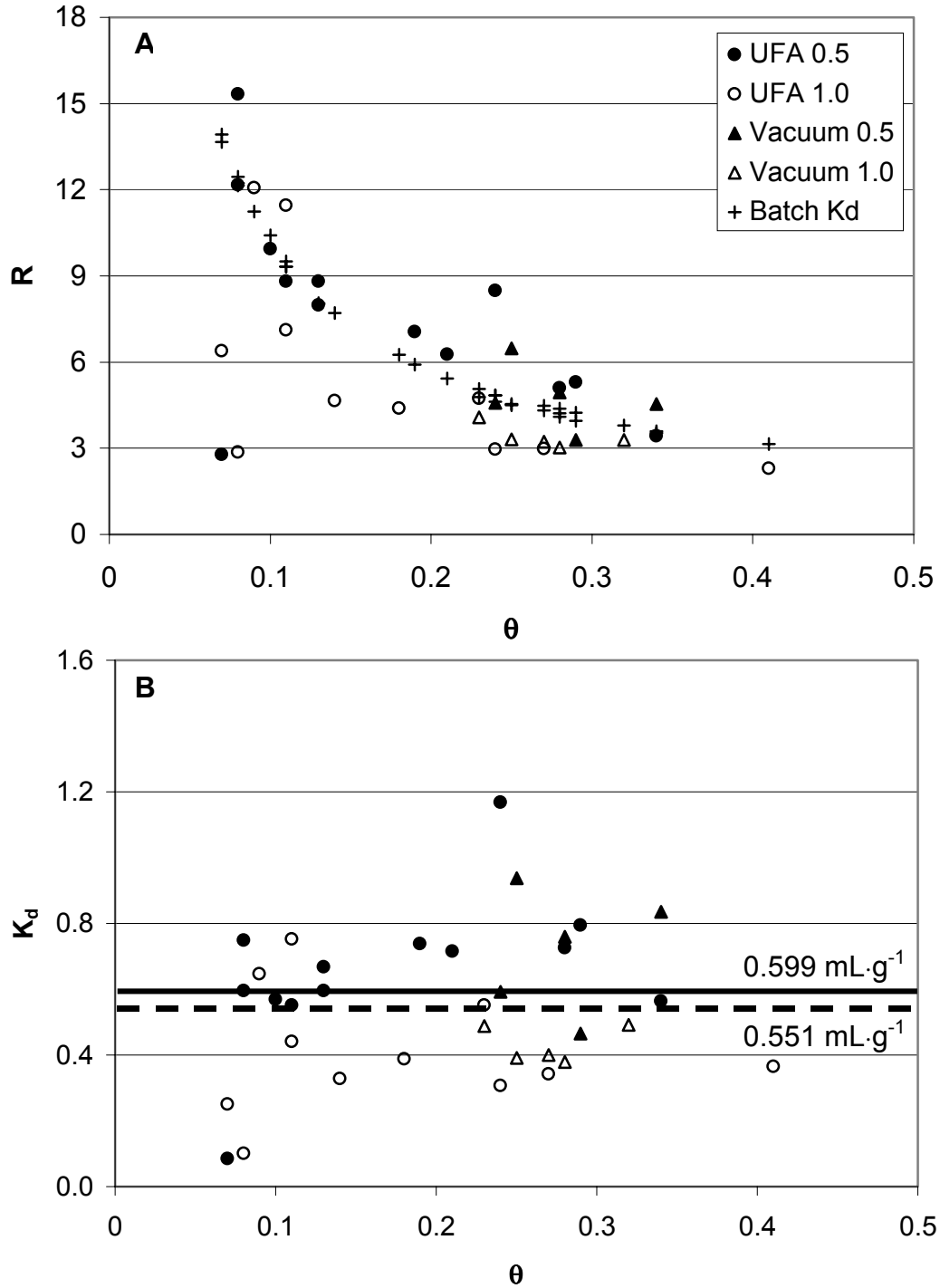


Figure 10. Retardation factor (graph A) and distribution coefficient (graph B) with volumetric water content for UFA and vacuum column systems at 0.5 mM and 1.0 mM Cr(VI) concentration levels. The batch K_d data in graph A are retardation factors calculated from the batch K_d sorption isotherm for each column experiment using the equation $R = 1 + [(\rho_b \cdot K_d) / \theta]$. The solid line in graph B represents the batch isotherm K_d and the dotted line represents the average of all column K_d values displayed in the graph.

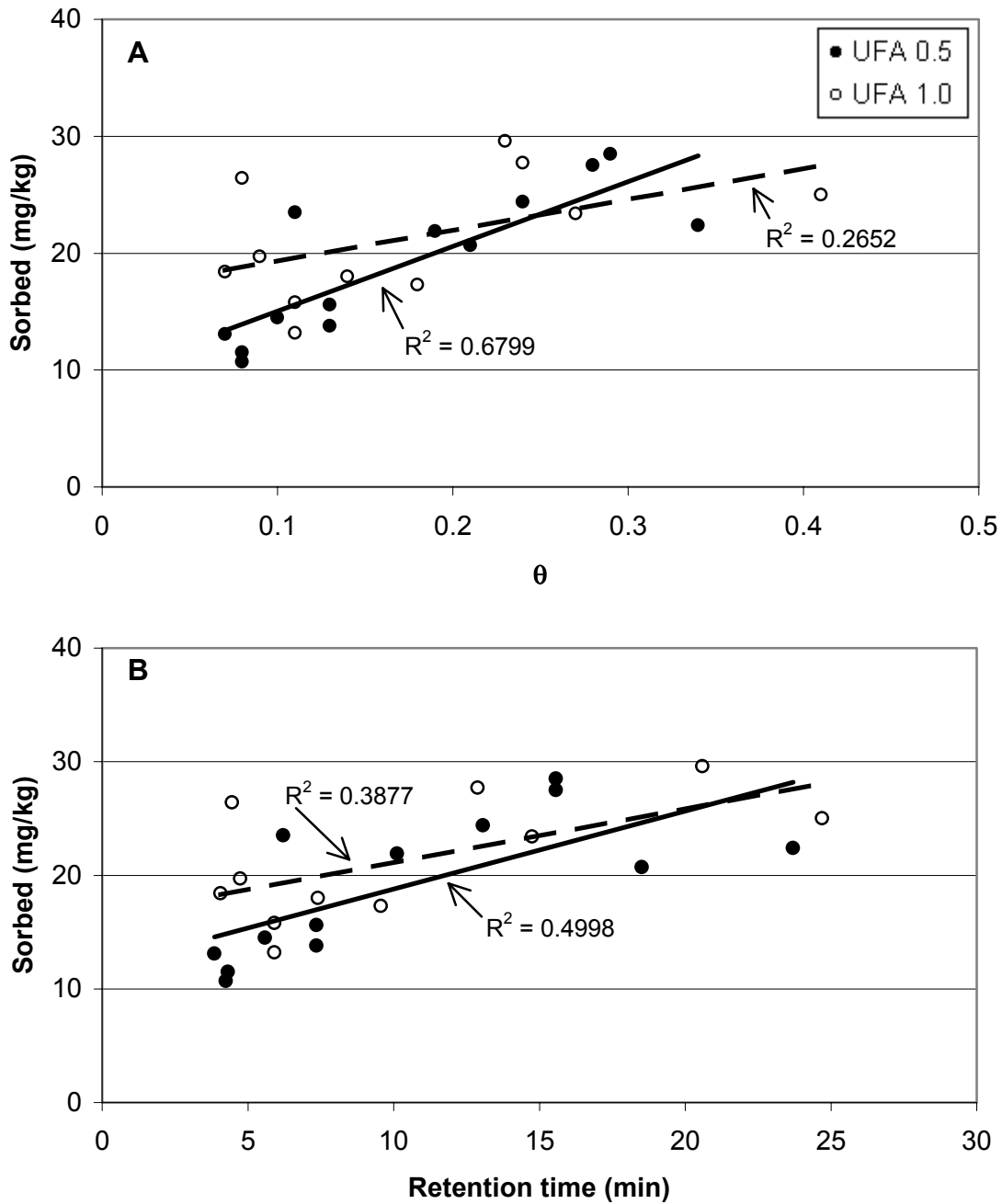


Figure 11. Amount of Cr(VI) sorbed by soil in UFA and vacuum column experiments with volumetric water content (graph A) and retention time of solute in soil column (graph B). Regression lines for the 0.5 mM (solid line) and 1.0 mM (dotted line) UFA data are shown.

CHAPTER 3

CONCLUSIONS

The objective of these solute transport experiments was to evaluate the effect of water content on the dispersion and sorption properties of tritium, bromide, and chromate in a sandy loam characteristic of the Atlantic Coastal Plain. Two methods were employed to create a steady-state unsaturated flow regime within the packed soil columns: a vacuum-based Wierenga column system and a centrifuge-based Unsaturated Flow Apparatus (UFA) column system. It was found that there was no significant difference in solute transport derived with these two systems. The experimental duration of the columns on the vacuum-based system were 4 to 23 times longer than those run with the centrifuge-based system at comparable water contents. The vacuum system was only able to reach water contents as low as 50% of saturation, while the UFA reached water contents as low as 16% of saturation. All UFA experiments could be finished in one day, though the system has to be stopped for sampling purposes at specified intervals.

Dispersion, as indicated by the Peclet number, increased with flow rate and decreasing water content. However, these relationships were not linear in nature. In general, retardation increased with decreasing water content, but this relationship was only significant for the UFA data. However, calculation of the distribution coefficient from the retardation factor indicated no trend with water

content. The average of all Cr(VI) column experiment K_d values, $0.551 \text{ mL}\cdot\text{g}^{-1}$, was similar to the batch sorption isotherm K_d value of $0.599 \text{ mL}\cdot\text{g}^{-1}$.

The total amount of Cr(VI) sorbed to the soil throughout the course of the experiment increased with increasing water content. However, Cr(VI) sorption also increased with the average solute retention time within the soil. It was unclear whether Cr(VI) sorption was influenced by changing solute flow paths with desaturation, or was a factor of average solute retention time.

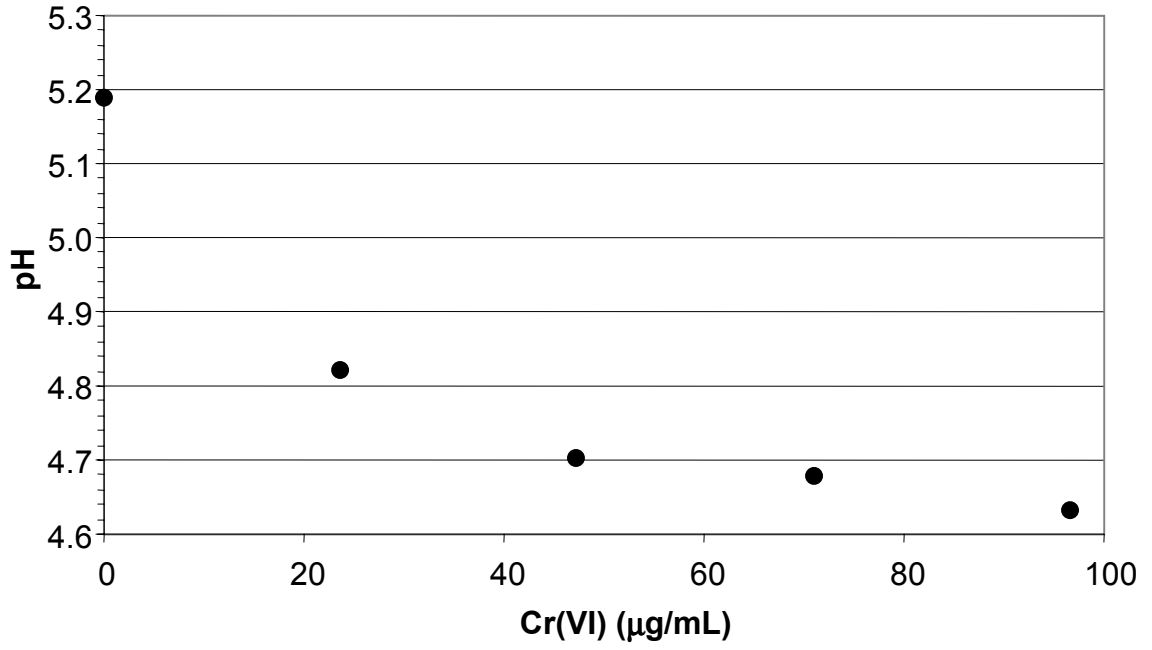
APPENDIX A – Water contents for vacuum-based solute transport columns and the flow rates and vacuum pressures used to obtain those water contents.

Water content (θ)	Flow rate (mL·hr⁻¹)	Vacuum pressure (cbar)
0.33	29	25
0.31	11	40
0.29	42	15
0.28	40	11
0.28	44	15
0.27	4	60
0.27	44	15
0.25	40	30
0.25	8	50
0.24	42	40
0.23	9	60

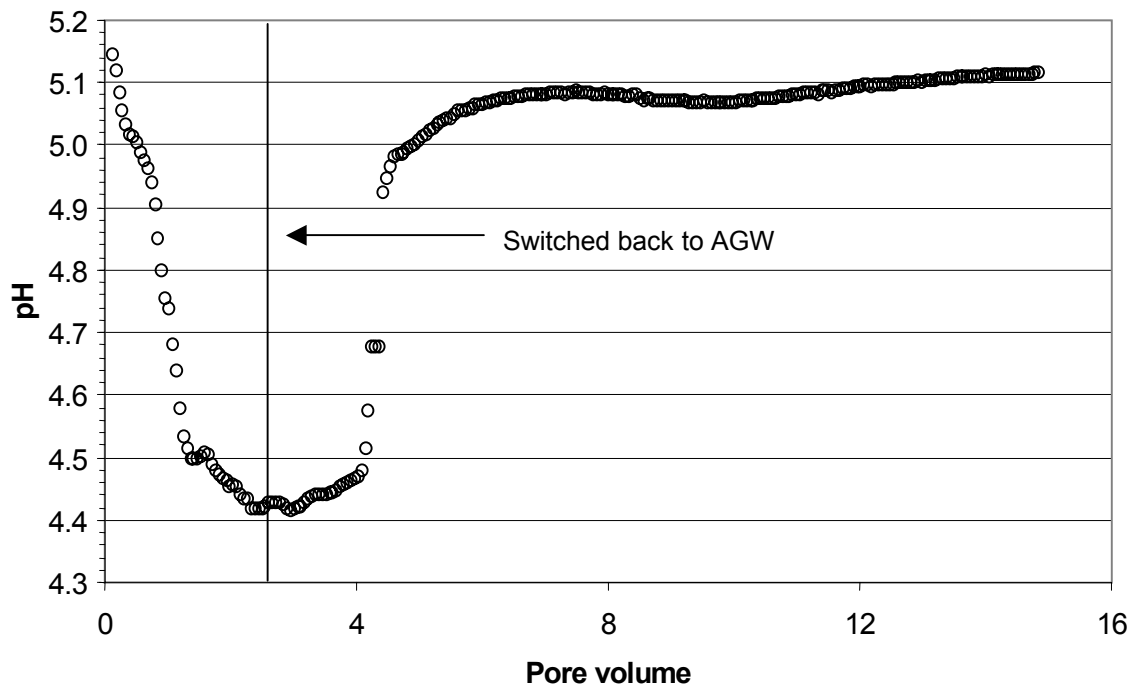
APPENDIX B – Water contents for the centrifuge-based solute transport columns (UFA) and the flow rates and centrifuge speeds used to obtain those water contents.

Water content (θ)	Flow rate (mL·hr⁻¹)	Centrifuge speed (rpm)
0.29	50	300
0.28	50	300
0.27	50	300
0.24	50	300
0.23	30	500
0.21	30	500
0.21	50	600
0.19	50	600
0.18	50	600
0.14	50	1000
0.13	50	1000
0.13	50	1000
0.11	50	1500
0.10	50	1500
0.09	50	1500
0.08	50	1500
0.07	50	1500

APPENDIX C – Cr(VI) sorption isotherm pH with Cr(VI) equilibrium solution concentration.



APPENDIX D – pH change during saturated experiment using 1 mM KBr solution at $0.694 \text{ cm}^3 \cdot \text{min}^{-1}$.



APPENDIX E – R² for regression values for parameters obtained using the equilibrium and nonequilibrium models[†]

Experiment [‡]	Equilibrium model R ²		Nonequilibrium model R ²	
	D and v	R	D, v, β, and ω	R
Br/V-1.0-78	0.9699	0.9853	0.9816	0.9824
Br/V-1.0-73	0.9939	0.9422	0.9939	0.9422
Br/V-1.0-68	0.9852	0.9862	0.9947	0.9822
Br/V-1.0-63	0.8984	0.9110	0.8977	0.9059
Cr/V-0.5-75	0.9655	0.5960	0.9654	0.6096
Cr/V-0.5-63	0.5496	0.9422	0.5484	0.9314
Cr/V-0.5-62	0.9934	0.5236	0.9934	0.5264
Cr/V-0.5-56	0.9141	0.7366	0.9141	0.7380
Cr/V-0.5-54	0.9793	0.8698	0.9793	0.8705
Cr/V-1.0-74	0.8772	0.8792	0.8770	0.8832
Cr/V-1.0-64	0.9678	0.4959	0.9675	0.5324
Cr/V-1.0-62	0.9912	0.8403	0.9912	0.8403
Cr/V-1.0-58	0.9963	0.9042	0.9963	0.9042
Cr/V-1.0-51	0.8933	0.7158	0.8915	0.7044
Cr/U-0.5-75	0.9708	0.9720	0.9702	0.9707
Cr/U-0.5-70	0.9426	0.9269	0.9426	0.6288
Cr/U-0.5-68	0.9623	0.4963	0.9597	0.6409
Cr/U-0.5-57	0.9460	0.9356	0.9460	0.9343
Cr/U-0.5-51	0.9722	0.6973	0.9722	0.6979
Cr/U-0.5-45	0.9940	0.8468	0.9940	0.8505
Cr/U-0.5-32	0.9960	0.4673	0.9982	0.4616
Cr/U-0.5-31	0.9799	0.5542	0.9794	0.4341
Cr/U-0.5-28	0.9879	0.7917	0.9879	0.7916
Cr/U-0.5-25	0.9777	0.8889	0.9779	0.9104
Cr/U-0.5-19	0.9660	0.8948	0.9655	0.8980
Cr/U-0.5-18	0.9919	0.7575	0.9918	0.7608
Cr/U-0.5-16	0.9942	0.7575	0.9942	0.7624
Cr/U-1.0-94	0.9546	0.9948	0.9544	0.9938
Cr/U-1.0-66	0.9782	0.0635	0.9781	0.2429
Cr/U-1.0-57a	0.9561	0.7685	0.9560	0.7610
Cr/U-1.0-57b	0.8497	0.9870	0.8490	0.9876
Cr/U-1.0-44	0.9925	0.9517	0.9925	0.9513
Cr/U-1.0-33	0.9885	0.8040	0.9885	0.8027
Cr/U-1.0-26a	0.9942	0.4181	0.9942	0.4194
Cr/U-1.0-26b	0.9947	0.2460	0.9946	0.2654
Cr/U-1.0-21	0.9210	0.8624	0.9201	0.8760
Cr/U-1.0-19	0.9942	0.9438	0.9941	0.8591
Cr/U-1.0-17	0.9703	0.7637	0.9703	0.7705

[†]Abbreviations are as follows: *D*, hydrodynamic dispersion coefficient; *v*, average pore water velocity; *R*, retardation factor; *β*, fraction of mobile water; and *ω*, mass transfer coefficient.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

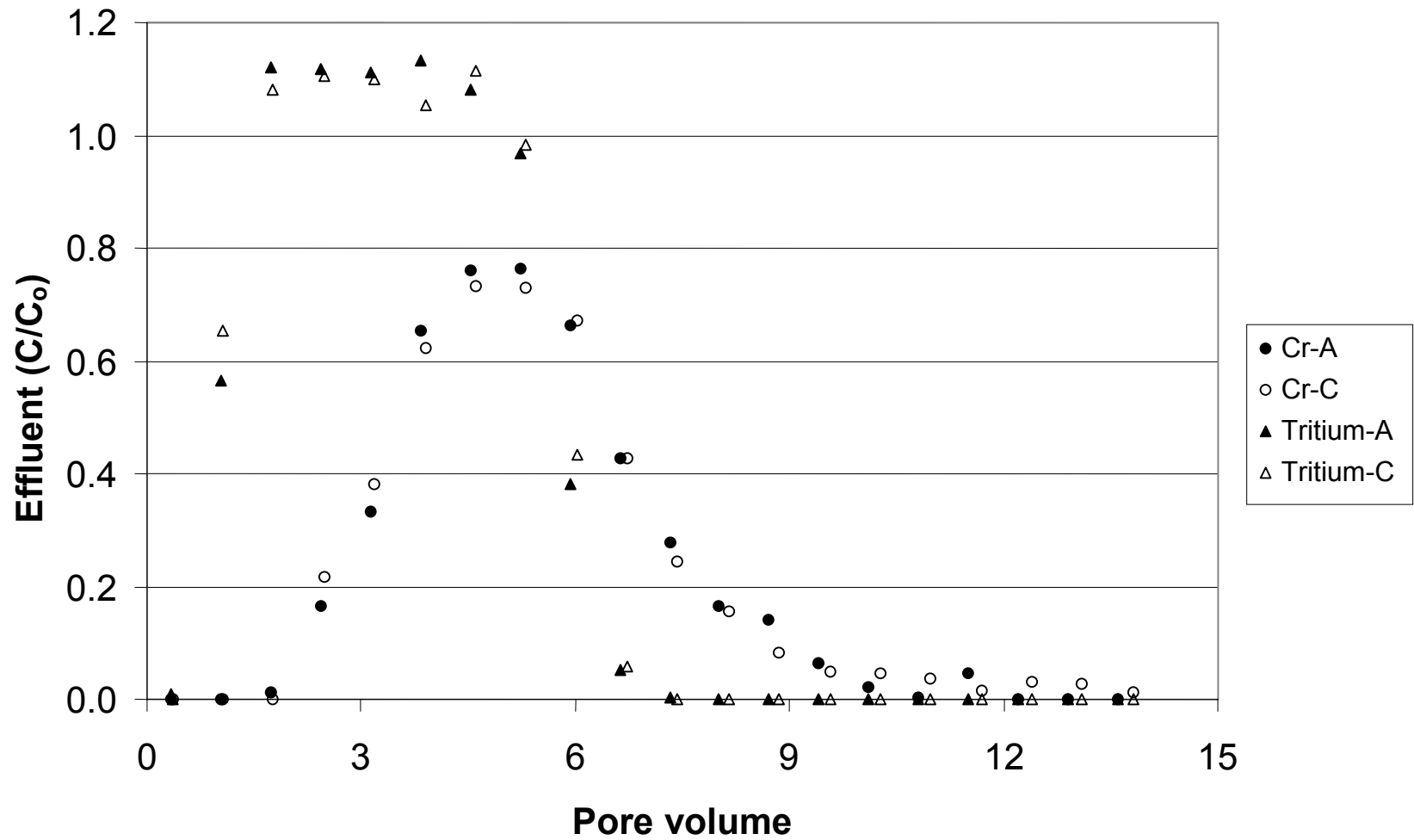
APPENDIX F – Comparison of bromide transport parameters determined using equilibrium and nonequilibrium models[†]

Experiment[‡]	v_{EQ} (cm·min⁻¹)	D_{EQ}	R_{EQ}	v_{NON} (cm·min⁻¹)	D_{NON}	R_{NON}	β
Br/V-1.0-78	0.4173	0.2182	1.289	0.4310	0.1000	1.343	0.9500
Br/V-1.0-73	0.3749	0.1348	1.347	0.3753	0.1349	1.348	0.9999
Br/V-1.0-68	0.1558	0.0641	1.393	0.1550	0.0889	1.387	0.9999
Br/V-1.0-63	0.0796	0.0719	1.595	0.0790	0.0909	1.595	0.9928

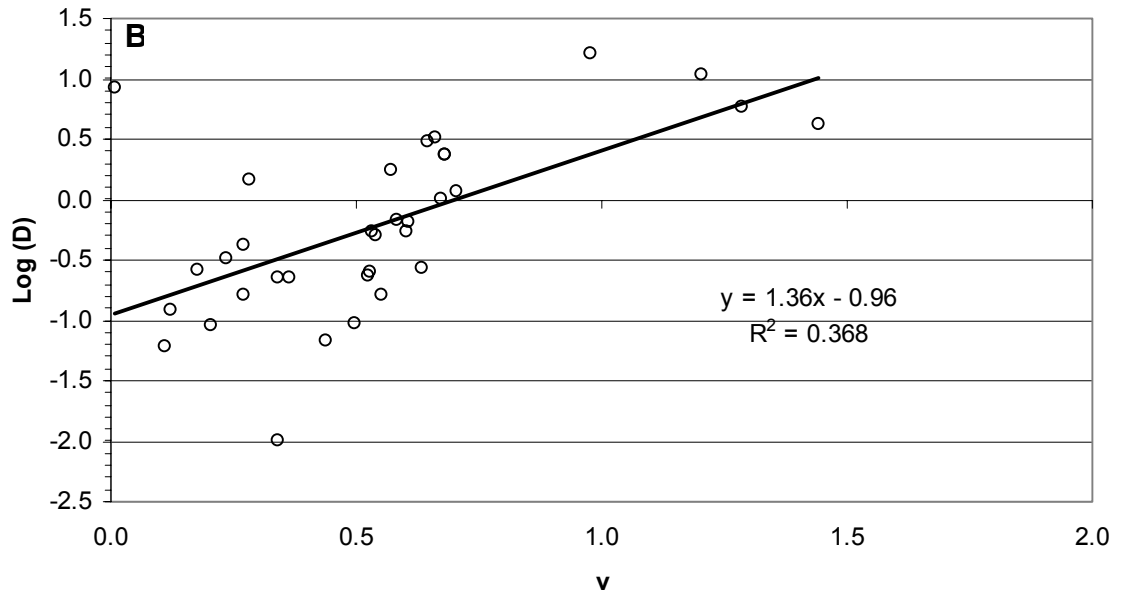
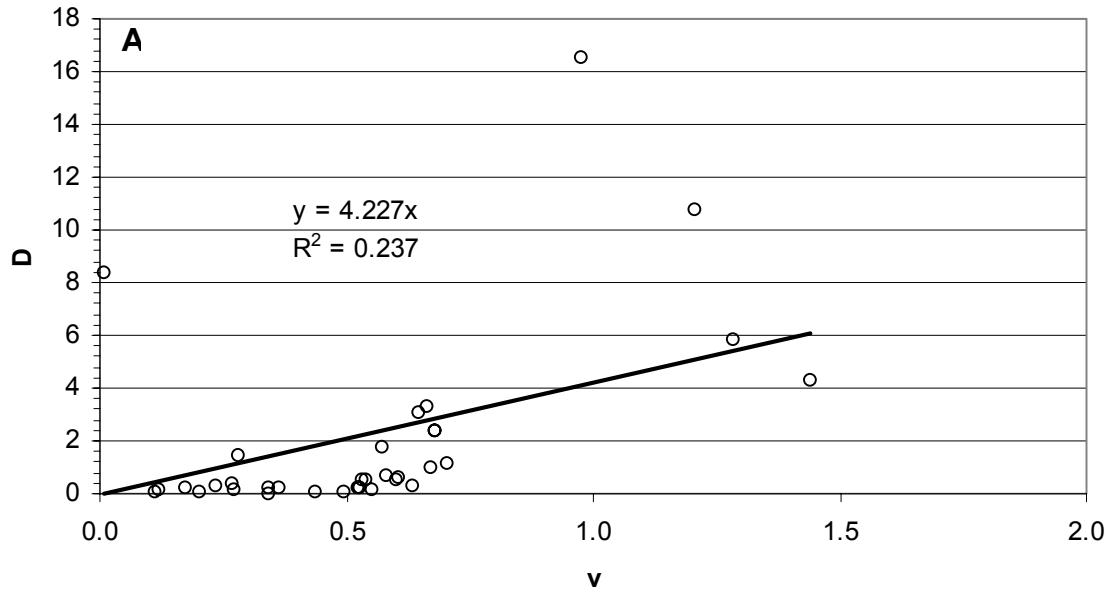
[†]Abbreviations are as follows: v , average pore water velocity; D , hydrodynamic dispersion coefficient; R , retardation factor; β , fraction of mobile water, and subscripts EQ and NON refer to equilibrium and nonequilibrium models, respectively.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

APPENDIX G – Simultaneous replicate (A and C) tritium and Cr(VI) breakthrough curves using the centrifuge-based solute transport column system (UFA). Results shown are for column Cr/U-1.0-66.



APPENDIX H -- Variation in D (graph A) and Log of D (graph B) with the average pore water velocity (v) for tritium data from vacuum and UFA column experiments.



APPENDIX I – Comparison of tritium transport parameters for vacuum system, determined from using the equilibrium (EQ) and nonequilibrium (NON) models[†]

Experiment[‡]	v_{EQ} ($\text{cm}\cdot\text{min}^{-1}$)	D_{EQ}	v_{NON} ($\text{cm}\cdot\text{min}^{-1}$)	D_{NON}	β
Cr/V-0.5-75	0.5221	0.2331	0.4651	0.2411	0.8903
Cr/V-0.5-63	0.2808	1.456	0.2750	1.721	0.9939
Cr/V-0.5-62	0.5317	0.5470	0.5321	0.5566	0.9999
Cr/V-0.5-56	0.1097	0.0619	0.1095	0.0629	0.9999
Cr/V-0.5-54	0.4360	0.0688	0.4351	0.0701	0.9991
Cr/V-1.0-74	0.6318	0.2718	0.6299	0.3048	0.9999
Cr/V-1.0-64	0.4950	0.0940	0.4929	0.1409	0.9987
Cr/V-1.0-62	0.5256	0.2570	0.5257	0.2572	0.9999
Cr/V-1.0-58	0.5510	0.1664	0.5516	0.1665	0.9999
Cr/V-1.0-51	0.1198	0.1223	0.1239	0.1160	0.9999

[†]Abbreviations are as follows: D , hydrodynamic dispersion coefficient; v , average pore water velocity; and β , fraction mobile water.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

APPENDIX J – Comparison of tritium transport parameters for the UFA (centrifuge system), 0.5 mM Cr(VI) tracer solution, determined using the equilibrium (EQ) and nonequilibrium (NON) models[†]

Experiment[‡]	v_{EQ} (cm·min⁻¹)	D_{EQ}	v_{NON} (cm·min⁻¹)	D_{NON}	β
Cr/U-0.5-75	0.2033	0.0916	0.2105	0.0905	0.9999
Cr/U-0.5-70	0.3622	0.2264	0.3306	0.2084	0.9141
Cr/U-0.5-68	0.3401	0.2276	0.3240	0.3303	0.9999
Cr/U-0.5-57	0.6052	0.6514	0.6026	0.6314	0.9931
Cr/U-0.5-51	0.2345	0.3335	0.2354	0.3359	0.9999
Cr/U-0.5-45	0.5381	0.5166	0.4817	0.4818	0.9999
Cr/U-0.5-32	0.6720	1.007	0.2778	0.3333	0.3947
Cr/U-0.5-31	0.5704	1.785	0.6127	1.544	0.9929
Cr/U-0.5-28	0.7020	1.188	0.7018	1.185	0.9996
Cr/U-0.5-25	0.0066	8.411	0.3513	7.933	0.9999
Cr/U-0.5-19	1.2040	10.78	1.152	11.24	0.9878
Cr/U-0.5-18	0.9767	16.56	0.9482	16.54	0.9974
Cr/U-0.5-16	0.6786	2.37	0.6765	2.466	0.9989

[†]Abbreviations are as follows: D , hydrodynamic dispersion coefficient; v , average pore water velocity; and β , fraction mobile water.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

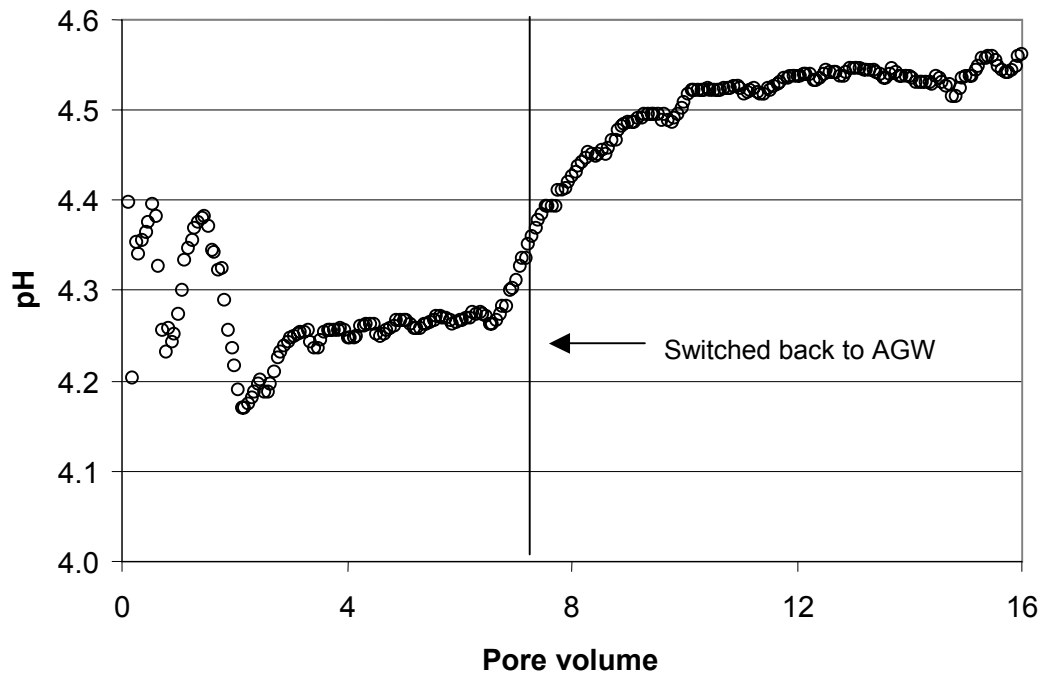
APPENDIX K – Comparison of tritium transport parameters for the UFA (centrifuge system), 1.0 mM Cr(VI) tracer solution, determined using the equilibrium (EQ) and nonequilibrium (NON) models[†]

Experiment[‡]	v_{EQ} (cm·min⁻¹)	D_{EQ}	v_{NON} (cm·min⁻¹)	D_{NON}	β
Cr/U-1.0-94	0.1741	0.2617	0.1751	0.2337	0.9999
Cr/U-1.0-66	0.3398	0.0101	0.3022	0.0262	0.8842
Cr/U-1.0-57a	0.2676	0.4168	0.2714	0.3973	0.9999
Cr/U-1.0-57b	0.2707	0.1623	0.2751	0.2236	0.9972
Cr/U-1.0-44	0.5807	0.6764	0.5789	0.6876	0.9999
Cr/U-1.0-33	0.6005	0.5527	0.6005	0.5484	0.9999
Cr/U-1.0-26a	0.6612	3.3190	0.6590	3.3570	0.9999
Cr/U-1.0-26b	0.6460	3.0920	0.5767	2.8440	0.9006
Cr/U-1.0-21	1.4420	4.2820	1.2250	4.9720	0.9758
Cr/U-1.0-19	0.6786	2.3700	0.6692	2.8770	0.9971
Cr/U-1.0-17	1.2840	5.8160	1.2750	6.0520	0.9999

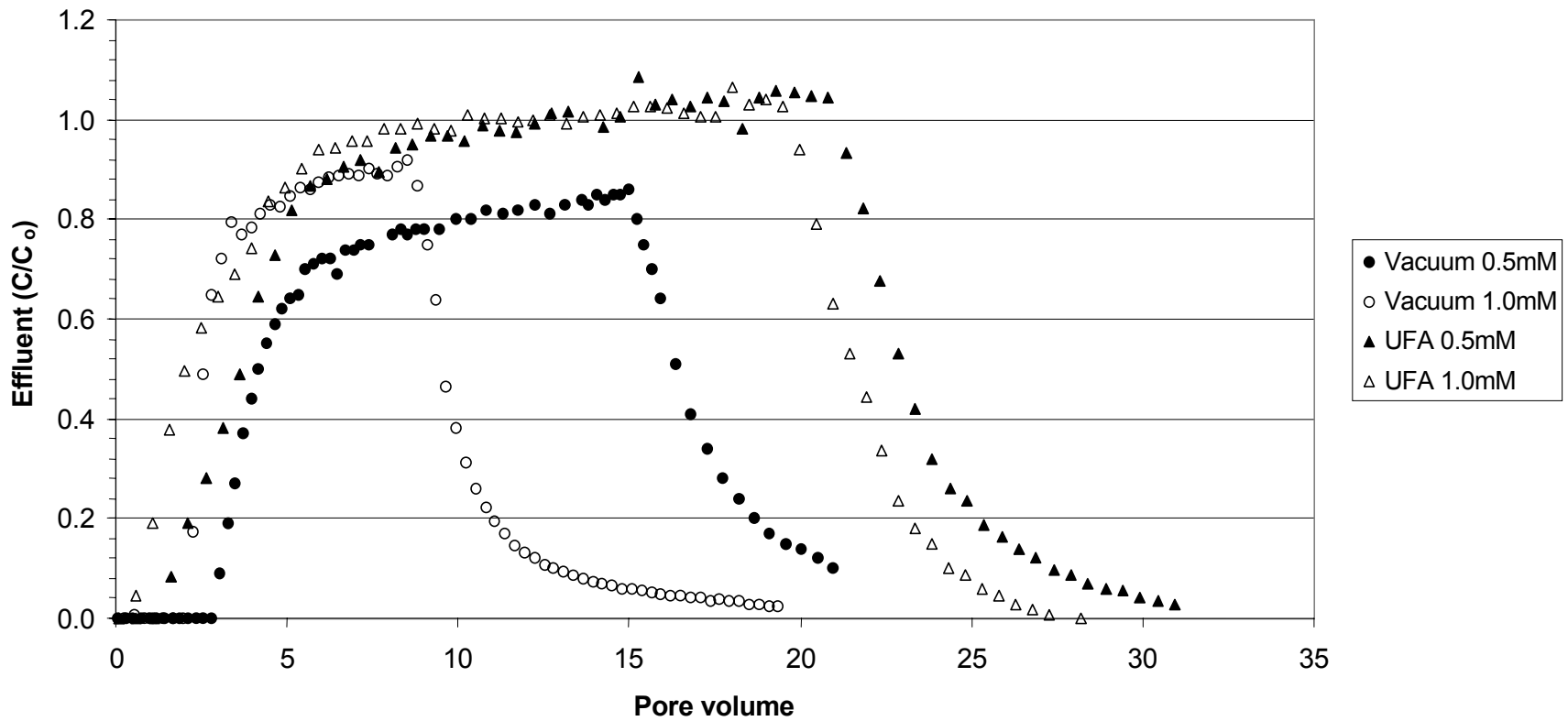
[†]Abbreviations are as follows: D , hydrodynamic dispersion coefficient; v , average pore water velocity; and β , fraction mobile water.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

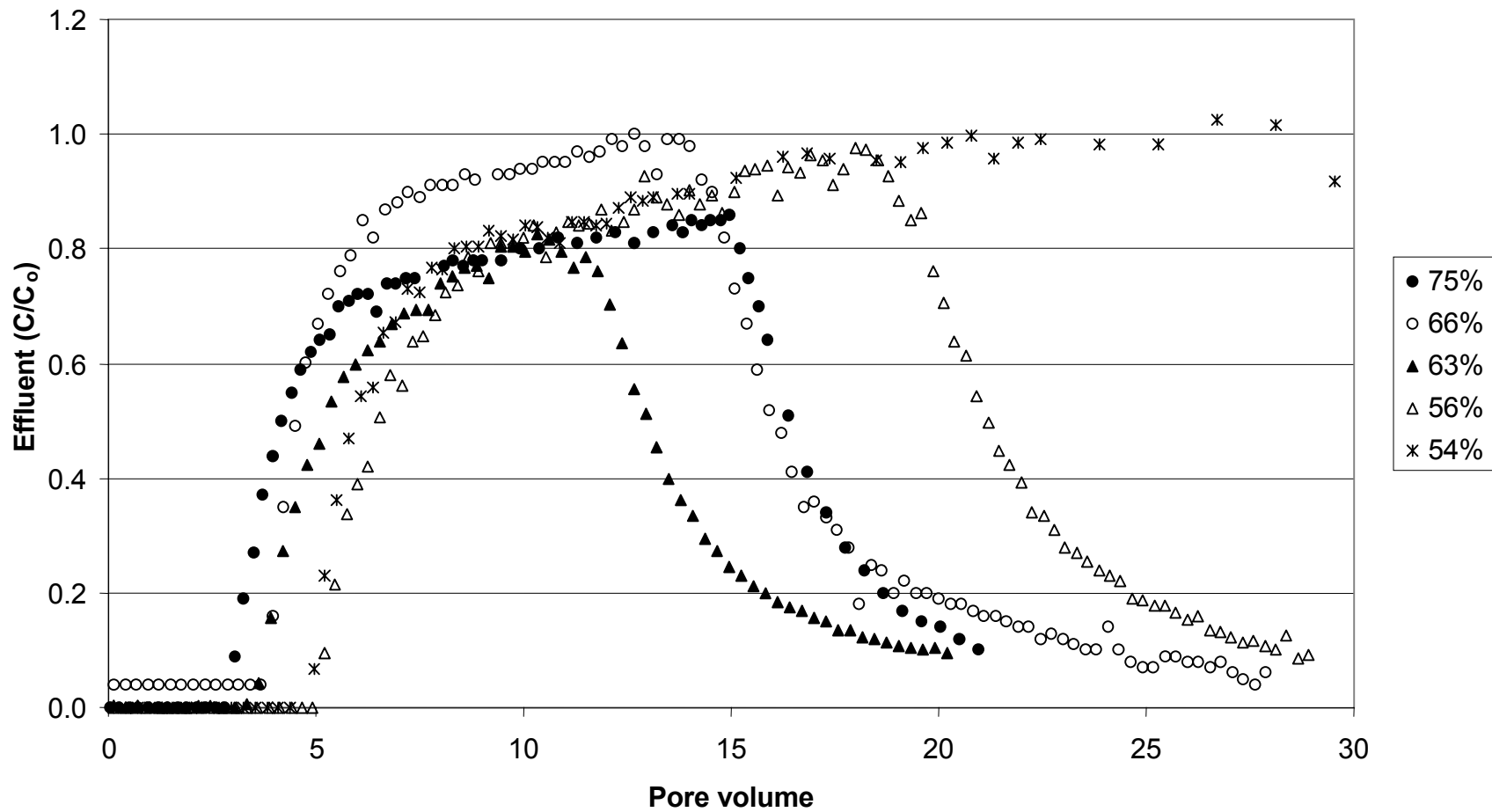
APPENDIX L – pH change during saturated Cr(VI) experiment using 1 mM $K_2Cr_2O_7$ solution at $0.789\text{ cm}^3\cdot\text{min}^{-1}$.



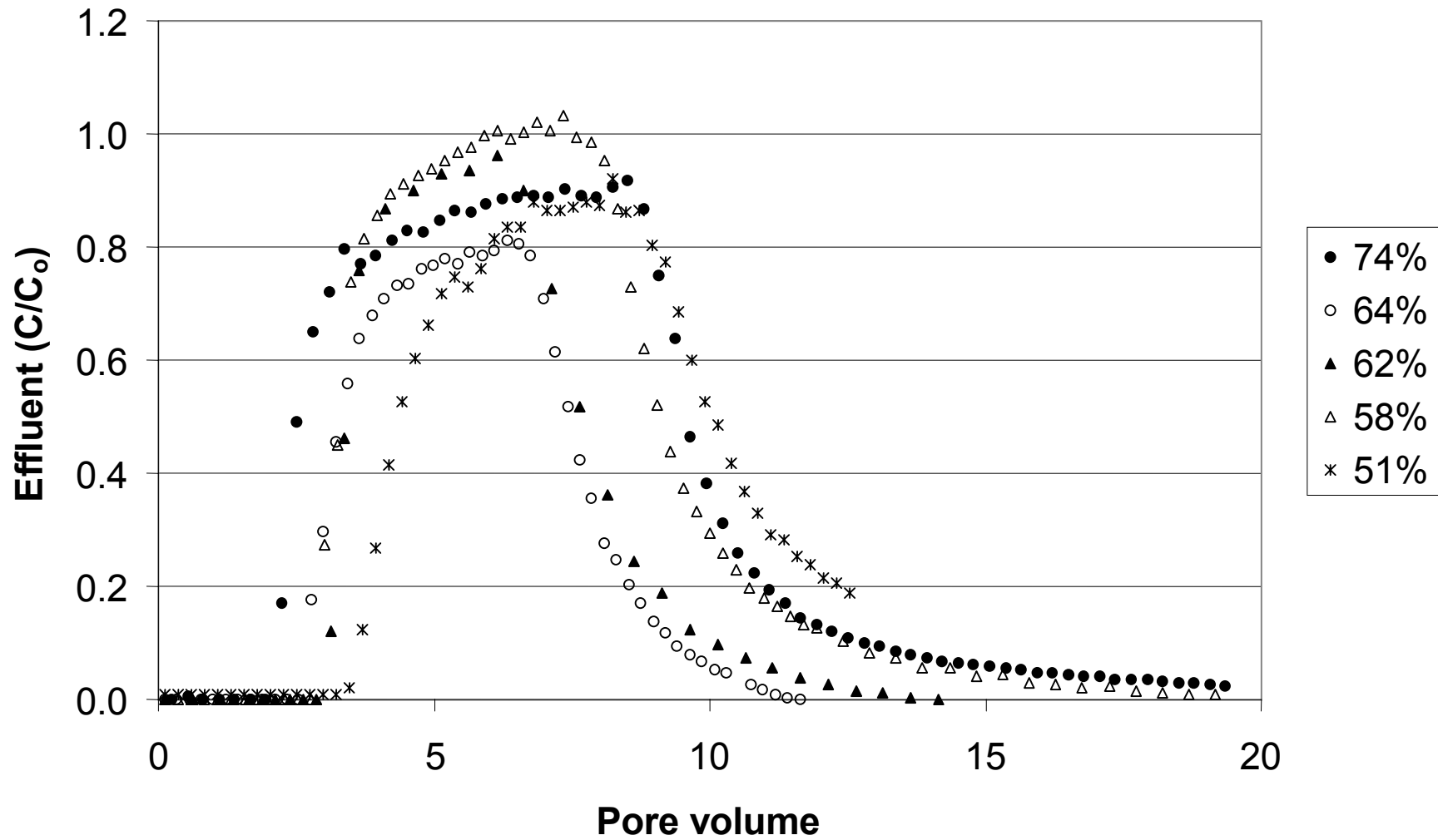
APPENDIX M – Saturated Cr(VI) breakthrough curves for vacuum and UFA experiments at 0.5 and 1.0 mM Cr(VI) concentrations.



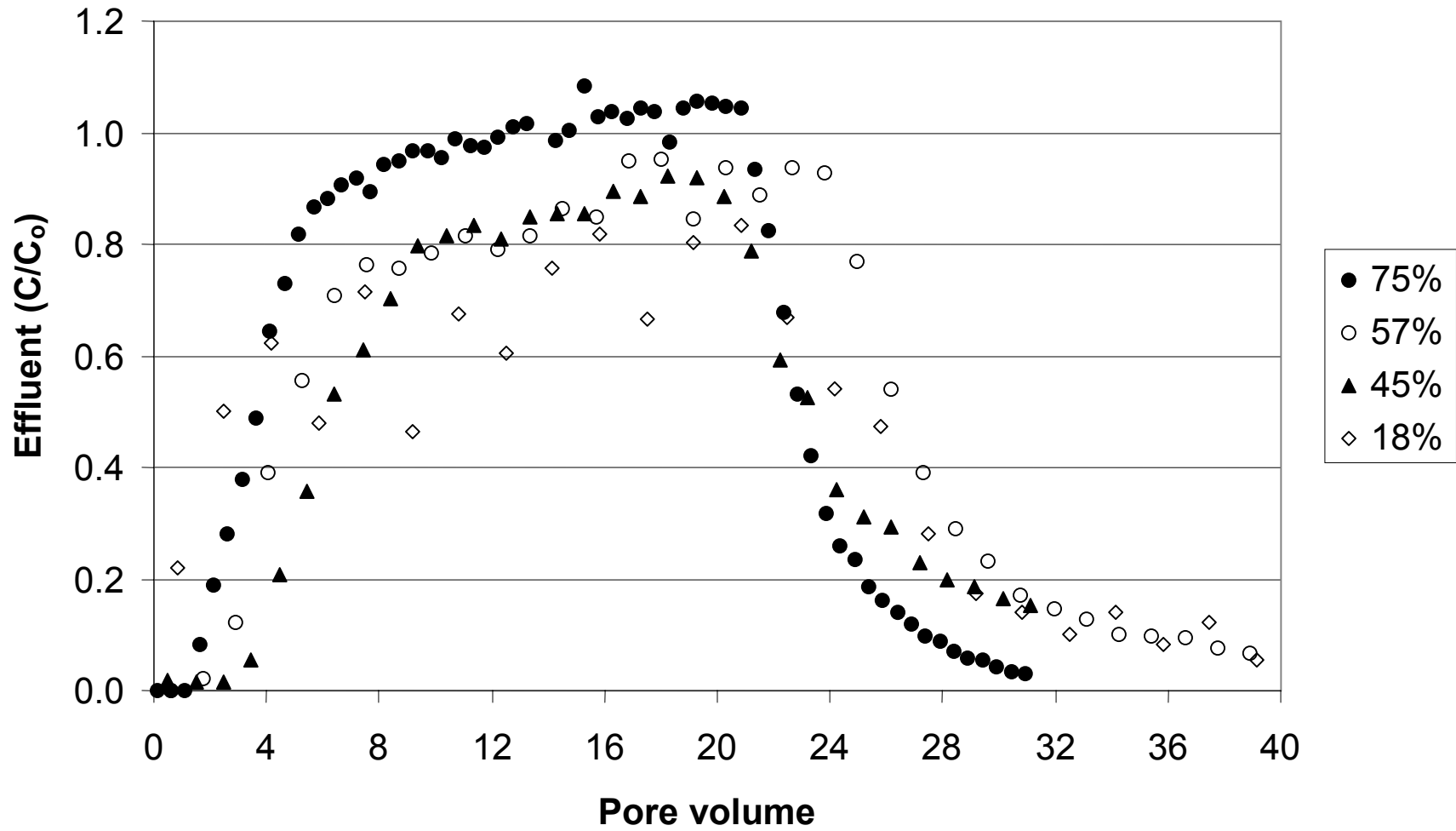
APPENDIX N – Cr(VI) vacuum curves for 0.5 mM Cr(VI) at all moisture saturation levels.



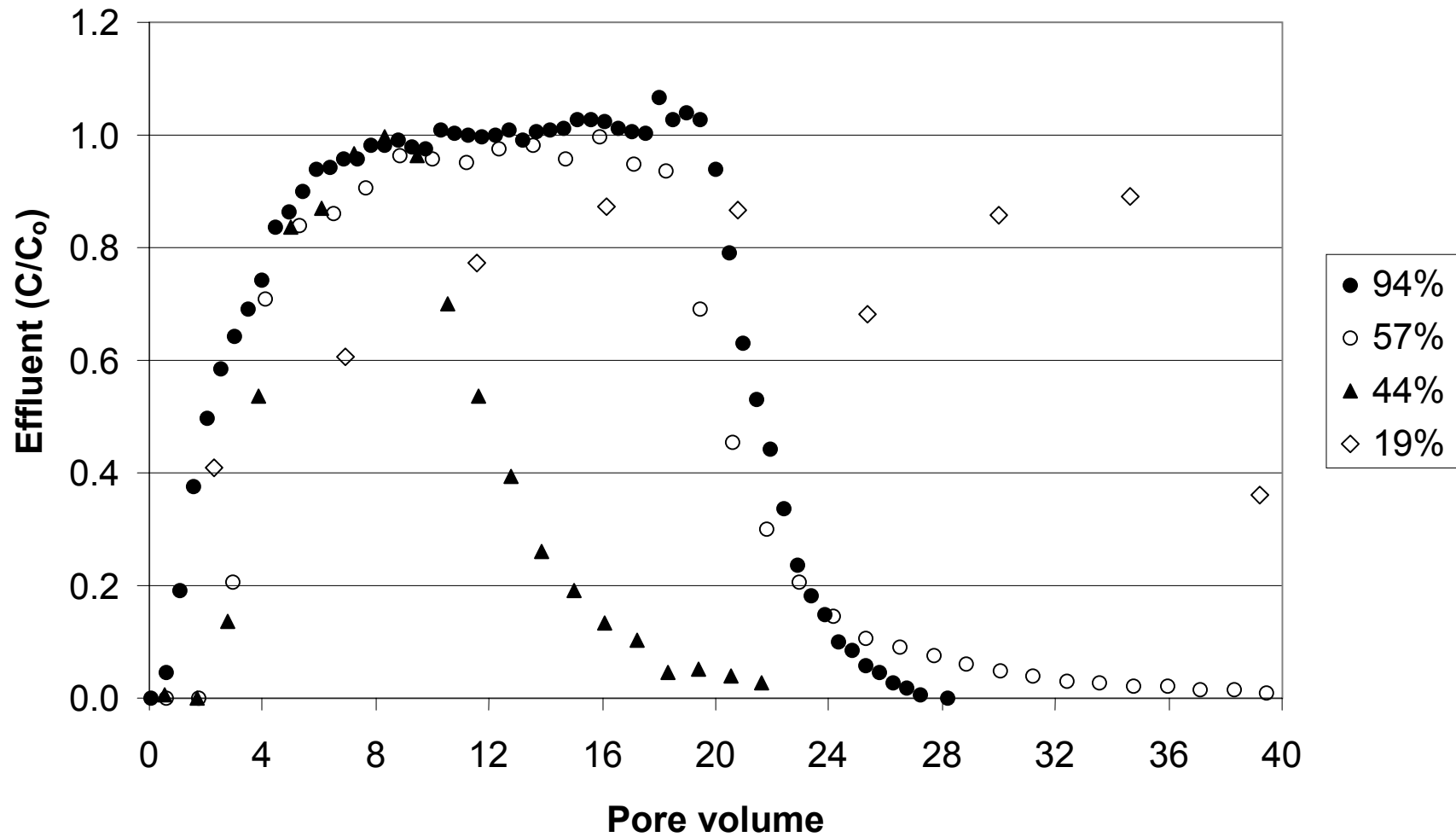
APPENDIX O – Cr(VI) vacuum curves for 1.0 mM Cr(VI) at all moisture saturation levels.



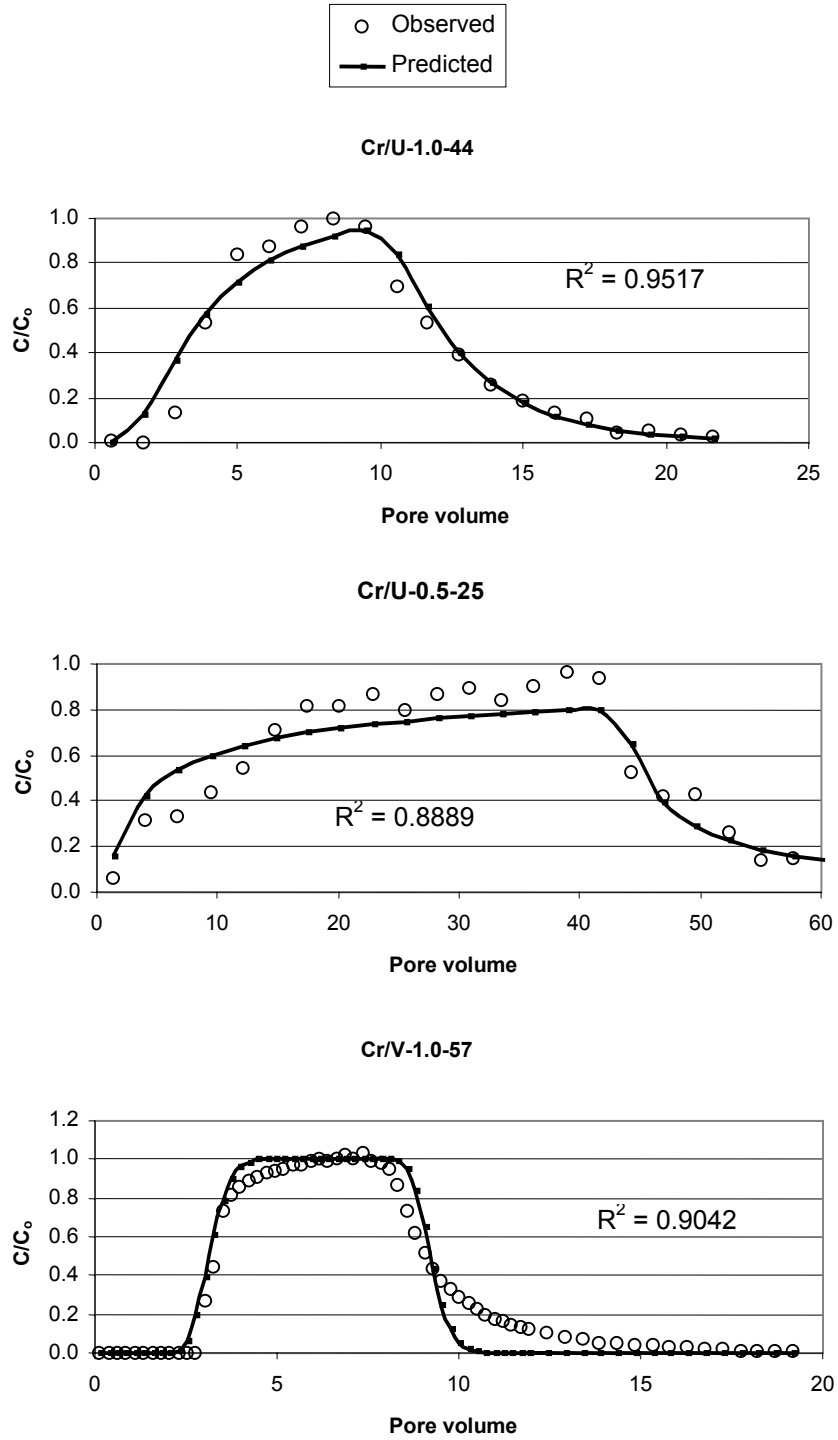
APPENDIX P – Selected Cr(VI) UFA 0.5mM curves at various moisture saturation levels.



APPENDIX Q – Selected Cr(VI) UFA 1.0 mM curves at various moisture saturation levels.



APPENDIX R – Observed and predicted Cr(VI) transport in UFA and vacuum columns. Curves are fit using the equilibrium transport model.



APPENDIX S – Comparison of Cr(VI) retardation factors for the vacuum system, determined by direct measurement or from solute breakthrough curves evaluated using the equilibrium (EQ) and nonequilibrium (NON) models[†]

Experiment[‡]	ρ_b (g·cm⁻³)	Q (cm³·min⁻¹)	θ	R_{EQ}	R_{NON}
Cr/V-0.5-75	1.44	0.813	0.34	4.540	4.571
Cr/V-0.5-63	1.43	0.706	0.29	3.296	3.317
Cr/V-0.5-62	1.45	0.735	0.28	4.938	4.946
Cr/V-0.5-56	1.46	0.131	0.25	6.476	6.469
Cr/V-0.5-54	1.45	0.700	0.24	4.578	4.575
Cr/V-1.0-74	1.49	0.789	0.32	3.284	3.285
Cr/V-1.0-64	1.50	0.671	0.28	3.032	3.034
Cr/V-1.0-62	1.50	0.730	0.27	3.222	3.222
Cr/V-1.0-58	1.48	0.664	0.25	3.314	3.317
Cr/V-1.0-51	1.45	0.142	0.23	4.074	4.207

[†]Abbreviations are as follows: ρ_b , bulk density; Q, flow rate; θ , volumetric water content; R_{EQ} , retardation coefficient from equilibrium model; and R_{NON} , retardation coefficient from nonequilibrium model.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

APPENDIX T – Comparison of Cr(VI) retardation factors for the UFA (centrifuge system), 0.5 mM Cr(VI) tracer solution, determined using the equilibrium (EQ) and nonequilibrium (NON) models[†]

Experiment [‡]	ρ_b (g·cm ⁻³)	Q (cm ³ ·min ⁻¹)	θ	R_{EQ}	R_{NON}
Cr/U-0.5-75	1.47	0.792	0.34	3.439	3.553
Cr/U-0.5-70	1.57	0.810	0.29	5.303	5.301
Cr/U-0.5-68	1.58	0.810	0.28	5.096	5.283
Cr/U-0.5-57	1.54	0.810	0.24	8.496	8.485
Cr/U-0.5-51	1.55	0.500	0.21	6.283	6.312
Cr/U-0.5-45	1.56	0.810	0.19	7.062	6.362
Cr/U-0.5-32	1.53	0.810	0.13	8.000	7.736
Cr/U-0.5-31	1.52	0.810	0.13	8.813	8.663
Cr/U-0.5-28	1.56	0.810	0.11	8.819	8.823
Cr/U-0.5-25	1.57	0.810	0.10	9.940	12.32
Cr/U-0.5-19	1.53	0.810	0.08	15.33	15.62
Cr/U-0.5-18	1.50	0.810	0.08	12.16	12.06
Cr/U-0.5-16	1.48	0.810	0.07	2.787	2.847

[†]Abbreviations are as follows: ρ_b , bulk density; Q, flow rate; θ , volumetric water content; R_{EQ} , retardation coefficient from equilibrium model; and R_{NON} , retardation coefficient from nonequilibrium model.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

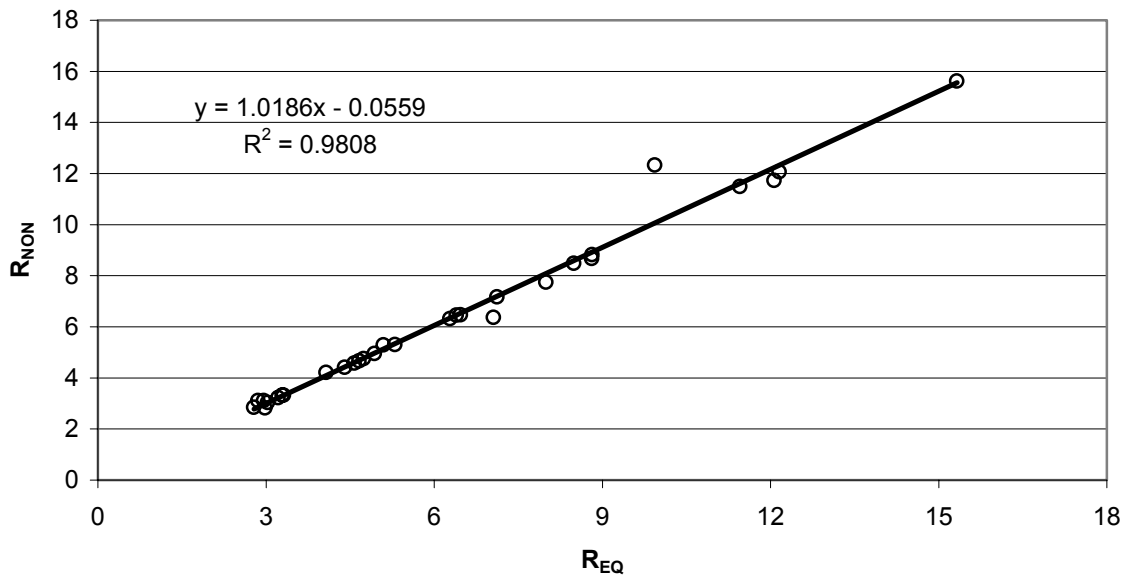
APPENDIX U – Comparison of Cr(VI) retardation factors for the UFA (centrifuge system), 1.0 mM Cr(VI) tracer solution, determined using the equilibrium (EQ) and nonequilibrium (NON) models[†]

Experiment [‡]	ρ_b (g·cm ⁻³)	Q (cm ³ ·min ⁻¹)	θ	R_{EQ}	R_{NON}
Cr/U-1.0-94	1.47	0.792	0.41	2.308	2.299
Cr/U-1.0-66	1.57	0.810	0.27	2.988	2.815
Cr/U-1.0-57a	1.56	0.500	0.23	4.745	4.746
Cr/U-1.0-57b	1.54	0.810	0.24	2.964	3.101
Cr/U-1.0-44	1.58	0.810	0.18	4.409	4.408
Cr/U-1.0-33	1.57	0.810	0.14	4.664	4.658
Cr/U-1.0-26a	1.53	0.810	0.11	11.46	11.49
Cr/U-1.0-26b	1.53	0.810	0.11	7.122	7.166
Cr/U-1.0-21	1.54	0.810	0.09	12.07	11.72
Cr/U-1.0-19	1.49	0.810	0.08	2.858	3.103
Cr/U-1.0-17	1.51	0.810	0.07	6.399	6.454

[†]Abbreviations are as follows: ρ_b , bulk density; Q, flow rate; θ , volumetric water content; R_{EQ} , retardation coefficient from equilibrium model; and R_{NON} , retardation coefficient from nonequilibrium model.

[‡]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.

APPENDIX V – Retardation factors calculated using the nonequilibrium model, R_{NON} , versus retardation factors calculated using the equilibrium model, R_{EQ} for UFA and vacuum Cr(VI) data.



APPENDIX W -- Percent recovery of Cr(VI) after experiment completion and after extraction with 10 mM KH₂PO₄ solution.

Experiment[†]	Experimental % recovery	% recovery after extraction
Cr/V-0.5-75	79.6	89.3
Cr/V-0.5-66	111.4	111.4
Cr/V-0.5-63	75.4	75.4
Cr/V-0.5-56	82.3	82.3
Cr/V-0.5-54	92.2	97.9
Cr/V-1.0-74	94.2	98.3
Cr/V-1.0-64	70.2	78.9
Cr/V-1.0-62	83.8	92.7
Cr/V-1.0-58	97.4	100.6
Cr/V-1.0-51	75.2	94.8
Cr/U-0.5-75	99.6	105.5
Cr/U-0.5-70	73.2	90.9
Cr/U-0.5-68	71.8	90.5
Cr/U-0.5-57	89.8	91.1
Cr/U-0.5-51	70.6	78.4
Cr/U-0.5-45	83.2	100.3
Cr/U-0.5-32	68.8	89.0
Cr/U-0.5-31	67.6	89.1
Cr/U-0.5-28	82.5	91.6
Cr/U-0.5-25	83.0	92.0
Cr/U-0.5-19	63.7	91.6
Cr/U-0.5-18	73.8	79.8
Cr/U-0.5-16	95.0	98.1
Cr/U-1.0-94	99.4	103.1
Cr/U-1.0-66	64.2	74.3
Cr/U-1.0-57a	77.9	86.2
Cr/U-1.0-57b	104.0	105.5
Cr/U-1.0-44	97.4	109.8
Cr/U-1.0-33	78.6	99.7
Cr/U-1.0-26a	53.2	77.1
Cr/U-1.0-26b	59.0	81.8
Cr/U-1.0-21	69.6	94.6
Cr/U-1.0-19	94.5	103.7
Cr/U-1.0-17	84.1	98.6
AVERAGE	81.4	92.5

[†]Experiments are identified by three components: column method (V = vacuum, U = UFA), tracer concentration (mM), and percent moisture saturation.